

DESIGN VALUES:		DESIGN VALUES ¹
DEAD AND LIVE LOADS		
ROOF LIVE LOAD		20 PSF
ROOF DEAD LOAD (SUPERIMPOSED ON FRAME) ²		LOAD SCENARIO=(1,2) DL= (3.5 PSF, 2 PSF)
ALLOWABLE SOIL PRESSURE^{3,5}		
SPREAD PAD		
VERTICAL BEARING: DL + Lr + SEISMIC (CONCRETE FOOTING)		1500 PSF
LATERAL COHESION: DL + Lr + SEISMIC (CONCRETE FOOTING)		130 PSF
DRILLED PIER		
SKIN FRICTION (DOWN): DL + Lr + SEISMIC (CONCRETE FOOTING) PER 1810A.3.3.1.4		167 PSF
SKIN FRICTION (UPLIFT): DL + Lr + SEISMIC (CONCRETE FOOTING) PER 1810A.3.3.1.5		83 PSF
LATERAL BEARING: DL + Lr + SEISMIC (CONCRETE FOOTING) PER 1810A.3.3.2		100 PSF/FT
ROOF SNOW LOAD⁴		
GROUND SNOW LOAD, P _g		10 PSF
RISK CATEGORY		III
ROOF SNOW LOAD: [] FLAT, Pf OR [] LOW SLOPE, Pm OR [X] SLOPED, Ps		11 PSF
SNOW ROOF SLOPE FACTOR, C _s		1.0
SNOW EXPOSURE FACTOR, C _e		1.2
SNOW LOAD IMPORTANCE FACTOR, I _s		1.1
THERMAL FACTOR, C _t		1.2
DRIFT SURCHARGE LOAD, P _d		0 PSF
DISTANCE FROM ADJACENT STRUCTURE, P _g = 0 PSF		4 IN
DISTANCE FROM ADJACENT STRUCTURE, P _g > 0 PSF		20 FT
ICE LOAD		0 PSF
FLOOD DESIGN		
FLOOD HAZARD AREA		NO
WIND DESIGN⁴		
BASIC WIND SPEED (3 SECOND GUST), V _{ult}		115 MPH
EXPOSURE CATEGORY		C
TOPOGRAPHIC FACTOR, K _z (1 MINIMUM)		1
INTERNAL PRESSURE COEFFICIENT, G _{cp} (IF APPLICABLE)		0.0
CLEAR WIND FLOW		YES
OBSTRUCTED WIND FLOW		YES
SEISMIC DESIGN⁴		
LATERAL FORCE-RESISTING SYSTEM		STEEL ORDINARY CANTILEVER COLUMN SYSTEM
ANALYSIS PROCEDURE		EQUIVALENT LATERAL FORCE PROCEDURE
SEISMIC DESIGN CATEGORY (SDC)		E
SEISMIC IMPORTANCE FACTOR, I _e		1.25
DESIGN BASE SHEAR, V		C _s x W
SEISMIC RESPONSE COEFFICIENT, C _s		LOAD SCENARIO = (1,2) C _s = (1.13, 1.65)
RESPONSE MODIFICATION FACTOR, R		1.25
SITE CLASS ⁷		E
REDUNDANCY FACTOR, p		1.3
MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _s - USED TO DETERMINE C _s		LOAD SCENARIO = (1,2) S _s = (1.406, 2.063)
SHORT-PERIOD SITE COEFFICIENT, F _a		1.2
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{ds} - USED TO DETERMINE C _s		LOAD SCENARIO = (1,2) S _{ds} (MAX) = (1.125, 1.650)
MAPPED SPECTRAL RESPONSE ACCELERATION AT 1 SECOND PERIOD, S ₁		LOAD SCENARIO = (1,2) S ₁ = (0.844, 1.07)
LONG-PERIOD SITE COEFFICIENT, F _v		2.0
DESIGN SPECTRAL RESPONSE ACCELERATION AT 1 SECOND PERIOD, S _{d1}		LOAD SCENARIO = (1,2) S _{d1} = (1.125, 1.427)
HORIZONTAL OR VERTICAL IRREGULARITIES TYPE(S)		NONE

- IF SITE-SPECIFIC DESIGN CRITERIA ARE OUTSIDE THE LIMITS OF THESE PC DRAWINGS, CONTACT POLYGON ENGINEERING TO SEE IF AN ENGINEERING LETTER, SUPPLEMENTAL DRAWINGS, AND/OR CALCULATIONS COULD BE SUBMITTED FOR A SITE-SPECIFIC SOLUTION. ANY SITE-SPECIFIC DEVIATION FROM THIS PC MAY NOT BE SUBMITTED TO DSA AS AN COVER- THE- COUNTRY
- STRUCTURE NOT DESIGNED TO SUPPORT SOLAR PANELS. STRUCTURE IS NOT DESIGNED TO SUPPORT SPRINKLER SYSTEMS IN LOAD SCENARIO 2 REGIONS
- GEHAZARD REPORTS ARE REQUIRED IF THE AREA COVERED UNDER THE ROOF EXCEEDS 4000 SQ FT, OR IS LOCATED WITHIN STATE OR LOCAL GEOLOGICAL HAZARD ZONE. VERIFY SUBMITTAL AND APPROVAL OF A GEHAZARD REPORT BY CGS PRIOR TO DSA SITE APPLICATION.
- STRUCTURAL SEPARATION BETWEEN ADJACENT STRUCTURES: MSL15= 2.75" MSL25= 2.75" MSL35= 2.5" STRUCTURAL SEPARATION BETWEEN EXISTING STRUCTURES: MSL15= 3.75" MSL25= 3.75" MSL35= 3.5"
- WHEN PLACING MULTIPLE CANOPIES WITH PIER FOOTINGS ADJACENT TO ONE ANOTHER, THE DESIGN MAY REQUIRE AN ANALYSIS OF GROUP EFFECTS ON THE FOUNDATIONS. THE MINIMUM CLEARANCE BETWEEN CENTER OF PIERS IS EIGHT TIMES PIER DIAMETER WITHOUT AN ACCOMPANYING ENGINEERING LETTER.
- SITE APPLICATION DESIGN PROFESSIONAL AND DSA REVIEWER SHALL VERIFY THE STRUCTURE TO BE LOCATED AT LEAST 20 FEET FROM ANY HIGHER ADJACENT STRUCTURE IF GROUND SNOW LOAD IS GREATER THAN ZERO.
- DESIGN COMPLIES WITH THE CONDITIONS OF EXCEPTION 1 OF ASCE 7-16 SECTION 11.4.8, ITEM 2.
- APPROVED FIRE APPARATUS ACCESS ROADS SHALL EXTEND TO WITHIN 150 FEET OF ALL PORTIONS OF THE PERIMETER OF THE STRUCTURE PER CFC 503.1.1.

ARCHITECTURAL REQUIREMENTS:

DESCRIPTION	DESIGN VALUES
TYPE OF CONSTRUCTION	II B
NUMBER OF STORIES	1
FIRE SPRINKLER SYSTEM	NOT BY POLYGON

RELATED BUILDING CODES AND STANDARDS: TITLE 24 CODES:

- 2022 CALIFORNIA ADMINISTRATIVE CODE (CAC) (PART 1, TITLE 24, CCR)
 2022 CALIFORNIA BUILDING CODE (CBC), VOLUMES 1 AND 2 (PART 2, TITLE 24, CCR)
 2021 INTERNATIONAL BUILDING CODE WITH 2022 CALIFORNIA AMENDMENTS)
 2022 CALIFORNIA ELECTRICAL CODE (PART 3, TITLE 24, CCR)
 2020 NATIONAL ELECTRICAL CODE WITH 2022 CALIFORNIA AMENDMENTS)
 2022 CALIFORNIA MECHANICAL CODE (CMC) (PART 4, TITLE 24, CCR)
 2021 INTERNATIONAL MECHANICAL CODE WITH 2022 CALIFORNIA AMENDMENTS)
 2022 CALIFORNIA PLUMBING CODE (CPC) (PART 5, TITLE 24, CCR)
 2021 UNIFORM PLUMBING CODE WITH 2022 CALIFORNIA AMENDMENTS)
 2022 CALIFORNIA ENERGY CODE (CEC) (PART 6, TITLE 24, CCR)
 2021 INTERNATIONAL FIRE CODE WITH 2022 CALIFORNIA AMENDMENTS)
 2022 CALIFORNIA EXISTING BUILDING CODE (CEBC) (PART 10, TITLE 24, CCR)
 2022 CALIFORNIA GREEN BUILDING STANDARDS CODE (CALGreen) (PART 11, TITLE 24, CCR)
 2022 CALIFORNIA REFERENCED STANDARDS CODE (PART 12, TITLE 24, CCR)
 TITLE 19 CCR, PUBLIC SAFETY, STATE FIRE MARSHAL REGULATIONS

- NFPA 13 - 2022
 NFPA 72 - 2022
- REFERENCE CODE SECTIONS FOR APPLICABLE STANDARDS:
- 2022 CBC, CHAPTER 35
 2022 CFC, CHAPTER 80

SCOPE OF WORK NARRATIVE:

THESE DRAWINGS ILLUSTRATE THE FABRICATION AND INSTALLATION REQUIREMENTS FOR A FREE-STANDING PREFABRICATED STEEL SHADE STRUCTURE. THE ENTIRE STRUCTURAL SYSTEM IS COMPRISED OF TUBULAR STEEL MEMBERS SUPPORTED ON CONCRETE FOUNDATIONS. THE FLEXIBILITY INCLUDED HEREIN ALLOWS THIS STRUCTURE TO COMPLY WITH A WIDE VARIETY OF PROJECT SITES AND LOADING REQUIREMENTS.

INSTRUCTIONS FOR ARCHITECTS PLANNING TO SUBMIT THESE PRE-CHECKED DRAWINGS TO DSA:

BEFORE SUBMITTING THESE PRE-CHECKED DRAWINGS FOR YOUR PROJECT, FOLLOW THE STEPS BELOW TO PROPERLY DEFINE THE APPROVED OPTIONS:
 THE POLYGON ENGINEERING DEPARTMENT IS AVAILABLE TO HELP YOU COMPLETE THESE STEPS (616) 888-3500

STEP 1 PROJECT INFORMATION	
PROJECT NAME	
SCHOOL DISTRICT	
USE AND OCCUPANCY CLASSIFICATION	(PROPOSED OCCUPANCY: A1, A2, A3, A4, A5, B, E)
OCCUPANT LOAD FACTOR	(15 SQFT/PERSON MAX; 5 SQFT/PERSON MIN FOR ANY A OCCUPANCY 20 SQFT/PERSON MAX FOR B OR E OCCUPANCY)
TOTAL ROOF AREA	
NUMBER OF OCCUPANTS	

STEP 2 DESIGN OPTIONS	
ROOF DECK	[] MULTI-RIB (MR) DEFAULT, WEIGHT 1.2 PSF [] STANDING SEAM (SS) WEIGHT 1.8 PSF
GUTTERS	[] NO DEFAULT [] YES SEE MSL7.0 FOR DETAILS
ELECTRICAL ACCESS	[] NO DEFAULT [] YES SEE MSL7.1 FOR DETAILS
CLEAR HEIGHT	[] 8' DEFAULT [] _____ OTHER 10' MAX

STEP 3 SEISMIC ACCELERATION	
S _s	(g)
S ₁	(g)

STEP 4 SEISMIC REGIONS			
0.000 < S _s <= 1.406	S ₁ <= 0.844	[] WHITE	3.5 PSF MAX DEAD LOAD
1.406 < S _s <= 2.063	S ₁ <= 1.070	[] GREEN	2.0 PSF MAX DEAD LOAD

STEP 5 TOTAL ROOF DEAD LOAD		
ROOF DECK	_____ PSF	SEE STEP 2 'ROOF DECK' FOR WEIGHTS
COLLATERAL	_____ PSF	LIGHTING, FIRE SUPPRESSION, ETC.
TOTAL	_____ PSF	ADD 'ROOF DECK' AND 'COLLATERAL'

STEP 6 LOAD SCENARIO	
WHITE	TOTAL ROOF DEAD LOAD <= 3.5 PSF [] LOAD SCENARIO 1
GREEN	TOTAL ROOF DEAD LOAD < 2.0 PSF [] LOAD SCENARIO 2

STEP 7 PC STRUCTURE	
ROOF WIDTH <= 15	[] MSL 15
15 < ROOF WIDTH <= 25	[] MSL 25
25 < ROOF WIDTH <= 35	[] MSL 35

STEP 8 STRUCTURE SIZE	
ROOF WIDTH	[] 15' DEFAULT [] 25' DEFAULT [] 35' DEFAULT [] _____ OTHER 6' MIN; 15' MAX [] _____ OTHER 15'-6" MIN; 25' MAX [] _____ OTHER 25'-6" MIN; 35' MAX
ROOF LENGTH	[] 144' 2 BAYS [] 144' 2 BAYS [] 144' 2 BAYS [] 164' 3 BAYS [] 164' 3 BAYS [] 164' 3 BAYS [] 184' 4 BAYS [] 184' 4 BAYS [] 184' 4 BAYS [] _____ OTHER [] _____ OTHER [] _____ OTHER

STEP 9 FOUNDATION TYPE		
MSL 15	MSL 25	MSL 35
[] SPREAD PAD [] DRILLED PIER	[] SPREAD PAD [] DRILLED PIER	[] SPREAD PAD

STEP 10 FOUNDATION SUMMARY		
MSL 15	MSL 25	MSL 35
[] LOAD SCENARIO 1 SPREAD PAD [] LOAD SCENARIO 1 DRILLED PIER [] LOAD SCENARIO 1 SPREAD PAD [] LOAD SCENARIO 1 DRILLED PIER [] LOAD SCENARIO 1 SPREAD PAD	[] LOAD SCENARIO 2 SPREAD PAD [] LOAD SCENARIO 2 DRILLED PIER [] LOAD SCENARIO 2 SPREAD PAD [] LOAD SCENARIO 2 DRILLED PIER [] LOAD SCENARIO 2 SPREAD PAD	[] LOAD SCENARIO 2 SPREAD PAD [] LOAD SCENARIO 2 DRILLED PIER [] LOAD SCENARIO 2 SPREAD PAD [] LOAD SCENARIO 2 DRILLED PIER [] LOAD SCENARIO 2 SPREAD PAD

STEP 11 SHEET INDEX			
BASE FRAME	MSL 15	MSL 25	MSL 35
ROOF DECK	MR	SS	MR
FOUNDATION TYPE	SPREAD PAD	DRILLED PIER	SPREAD PAD
SELECT ONE	[] [] [] [] [] [] [] [] [] [] [] []		
ORDER FORM	MSL1.0	MSL1.0	MSL1.0
NOTES AND SPECIAL INSPECTIONS	MSL1.1	MSL1.1	MSL1.1
FOUNDATION PLAN	MSL2.0	MSL2.1	MSL2.2
FRAMING PLAN	MSL3.0	MSL3.0	MSL3.1
FRAME CONNECTION DETAILS	MSL4.0	MSL4.0	MSL4.1
ARCHITECTURAL VIEWS	MSL5.0	MSL5.0	MSL5.1
ROOF CONNECTION DETAILS	MSL6.0	MSL6.0	MSL6.1
MISC DESIGN OPTIONS	MSL7.0	MSL7.0	MSL7.0
ELECTRICAL CUTOUPS	MSL7.1	MSL7.1	MSL7.1

STEP 12 MULTIPLE STRUCTURES		
MULTIPLE STRUCTURES	ROOF WIDTH X LENGTH	QTY

- STEP 1: GENERAL PROJECT INFORMATION**
- IDENTIFY PROJECT NAME AND SCHOOL DISTRICT
 - IDENTIFY USE AND OCCUPANCY CLASSIFICATION
 - THE USE AND OCCUPANCY DETERMINE THE MAXIMUM SQUARE FOOTAGE OF THE STRUCTURE
 - THE MAXIMUM SQUARE FOOTAGE IS ALSO LIMITED BY THE NUMBER OF OCCUPANTS
 - IDENTIFY THE OCCUPANT LOAD PER TABLE 1006 IN THE CBC
 - IDENTIFY TOTAL ROOF AREA WHICH SHALL NOT EXCEED ALLOWABLE AREA PER TABLE 506.2 IN THE CBC.
 - IDENTIFY EXPECTED NUMBER OF OCCUPANTS BASED ON THE ESTIMATED OCCUPANT LOAD
 - TOTAL ROOF AREA DIVIDED BY OCCUPANT LOAD CAN DETERMINE NUMBER OF OCCUPANTS

- STEP 2: DESIGN OPTIONS**
- SELECT ROOF DECK FOR YOUR PROJECT
 - "MR" REPRESENTS MCELROY METAL "MULTI-RIB" ROOF DECK
 - "SS" REPRESENTS MCELROY METAL "MEDALLION-LOCK" 16" STANDING SEAM ROOF DECK
 - SELECT WHETHER GUTTERS AND DOWNSPOUTS FROM POLYGON IS NEEDED FOR YOUR PROJECT
 - IF "YES", THEN INCLUDE SHEET MSL7.0 IN THE DRAWING SET
 - SELECT WHETHER ELECTRICAL CUTOUPS ARE NEEDED FOR YOUR PROJECT
 - SHEET MSL7.0 SHOWS ELECTRICAL CUTOUT SIZE AND LOCATION CUTOUPS IN COLUMNS
 - SHEET MSL7.1 HAS INSTRUCTIONS AND SHEET TO IDENTIFY WHICH COLUMNS
 - SHEET MSL7.1 MUST BE FILLED OUT IN THE SUBMITTAL SET APPROVED BY DSA
 - IF NOTHING IS FILLED IN ON MSL7.1, POLYGON WILL ASSUME CUTOUPS ARE ONLY IN COLUMN A1 (SEE 'FRAMING PLAN' FOR REFERENCE)
 - SELECT CLEAR HEIGHT (SEE 'ARCHITECTURAL VIEWS' SHEET FOR REFERENCE)
 - IF NOTHING IS SELECTED, POLYGON WILL ASSUME THE DEFAULT FOR EACH DESIGN OPTION

- STEP 3: IDENTIFY THE S_s & S₁ ACCELERATION (g) FOR YOUR PROJECT AND GEOTECHNICAL INFORMATION**
- S_s & S₁ VALUE DETERMINES THE REQUIRED SEISMIC DESIGN FORCES
 - S_s & S₁ VALUE DEPENDS ON PROJECT'S GEOGRAPHICAL LOCATION
 - FIND S_s & S₁ VALUES FOR YOUR PROJECT IN THE SITE SPECIFIC GEOTECHNICAL REPORT
 - FIND S_s & S₁ VALUES FOR YOUR PROJECT USING <https://asce7.hazardtool.com/>
 - THIS PC IS NOT APPROVED FOR S_s VALUES GREATER THAN 2.063 (CONTACT POLYGON FOR ADDITIONAL OPTIONS)

- STEP 4: IDENTIFY THE SEISMIC REGION FOR YOUR PROJECT**
- THE REGIONS ARE DEPENDANT ON THE S_s & S₁ VALUES DETERMINED IN STEP 3
 - THE SEISMIC REGION DICTATES THE MAXIMUM DEAD LOAD PERMITTED (SEE TABLE TO THE LEFT)
- STEP 5: IDENTIFY THE ROOF DEAD LOAD FOR YOUR PROJECT**

- THE ROOF DECK DEAD LOAD WILL ALWAYS BE INCLUDED
- THE COLLATERAL LOAD REPRESENTS ADDITIONAL LOAD THAT CAN BE SUPPORTED BY THE FRAME
- TOTAL ROOF DEAD LOAD MUST BE LESS THAN OR EQUAL TO THE MAX DEAD LOAD SHOWN IN STEP 4
- CUT SHEETS OF ANY BOARDS, BOXES AND EQUIPMENT TO BE MOUNTED ON THE STRUCTURE, INCLUDING WEIGHTS AND DIMENSIONS ARE REQUIRED

- STEP 6: IDENTIFY THE LOAD SCENARIO**
- REFERENCE THE STEP 4 COLOR AND SELECT THE APPLICABLE LOAD SCENARIO
 - LOAD SCENARIOS HAVE NO IMPACT ON FRAME DESIGN OR COST, BUT DO AFFECT FOUNDATION SIZE

- STEP 7: IDENTIFY PC STRUCTURE**
- ROOF WIDTHS UP TO 15' WIDE USE THE "MSL 15"
 - ROOF WIDTHS UP TO 25' WIDE USE THE "MSL 25"
 - ROOF WIDTHS UP TO 35' WIDE USE THE "MSL 35"
 - THE 15', 25', AND 35' WIDTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST ECONOMICAL
 - MAXIMUM WIDTH IS 35'; (SEE 'ARCHITECTURAL VIEWS' SHEET FOR REFERENCE)

- STEP 8: IDENTIFY SITE SPECIFIC ROOF WIDTH AND LENGTH**
- DO NOT EXCEED THE TOTAL ROOF AREA FROM STEP 1 (ROOF WIDTH MULTIPLIED BY ROOF LENGTH)
- STEP 9: FOUNDATION TYPE**
- SELECT A FOUNDATION BASED THE DESIRED FOUNDATION TYPE
 - SELECT EITHER SPREAD PAD OR DRILLED PIER FOUNDATION PRIOR TO APPROVAL
 - FOUNDATION TYPE IMPACTS CONSTRUCTION TIMING, SEQUENCE, COST, ETC.
 - FOUNDATION TYPE IMPACTS ANCHOR BOLT LENGTH (NOT PROVIDED BY POLYGON)
 - REVIEW OF SITE-SPECIFIC SOILS REPORT TO EVALUATE APPLICABILITY OF FOUNDATION OPTIONS AVAILABLE

- STEP 10: FOUNDATION SUMMARY**
- USE THE SELECTIONS FROM STEP 6 AND STEP 9 TO SELECT THE APPROPRIATE FOUNDATION
- STEP 11: SELECT APPLICABLE SHEET INDEX FOR YOUR PROJECT**

- IDENTIFY THE APPLICABLE SHEET INDEX
- INCLUDE APPLICABLE SHEET WITH YOUR DSA SUBMITTAL
- EXCLUDE 'MISC DESIGN OPTIONS' SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUPS OR GUTTERS
- EXCLUDE 'ELECTRICAL CUTOUPS' SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUPS

- STEP 12: MULTIPLE STRUCTURES WITH THE SAME PC#**
- FILL IN ROOF LENGTH AND WIDTH OF STRUCTURES AS WELL AS QUANTITY
 - UNO ON THE POLYGON DRAWINGS, POLYGON WILL ASSUME ALL DESIGN CRITERIA FOR EACH STRUCTURE IS THE SAME
 - CONTACT POLYGON FOR FURTHER INFORMATION

- STEP 13: COLUMN BASE PROTECTION**
- SELECT THE METHOD OF COLUMN INSTALLATION ON APPLICABLE FOUNDATION PLAN SHEET, DETAIL 2, SHEET 3.

- STEP 14: IDENTIFICATION STAMP**
- IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT
 - APP: 02-121212 PC
 - REVIEWED FOR
 - SS FLS ACS CG
 - DATE: 7/18/2023

SHEET INDEX			
1 MSL1.0	ORDER FORM	11 MSL4.0	FRAME CONNECTION DETAILS- MSL 15
2 MSL1.1	NOTES AND SPECIAL INSPECTIONS	12 MSL4.1	FRAME CONNECTION DETAILS- MSL 25
3 MSL2.0	FOUNDATION PLAN SPREAD PAD- MSL 15	13 MSL4.2	FRAME CONNECTION DETAILS- MSL 35
4 MSL2.1	FOUNDATION PLAN DRILLED PIER- MSL 15	14 MSL5.0	ARCHITECTURAL VIEWS- MSL 15
5 MSL2.2	FOUNDATION PLAN SPREAD PAD- MSL 25	15 MSL5.1	ARCHITECTURAL VIEWS- MSL 25
6 MSL2.3	FOUNDATION PLAN DRILLED PIER- MSL 25	16 MSL5.2	ARCHITECTURAL VIEWS- MSL 35
7 MSL2.4	FOUNDATION PLAN SPREAD PAD- MSL 35	17 MSL6.0	ROOF CONNECTION DETAILS
8 MSL3.0	FRAMING PLAN- MSL 15	18 MSL6.1	ROOF CONNECTION DETAILS
9 MSL3.1	FRAMING PLAN- MSL 25	19 MSL7.0	MISC DESIGN OPTIONS
10 MSL3.2	FRAMING PLAN- MSL 35	20 MSL7.1	ELECTRICAL CUTOUPS
TOTAL SHEETS = 20			

ABBREVIATIONS:

ACI	AMERICAN CONCRETE INSTITUTE	MR	MULTI-RIB ROOF PANEL (MCELROY)
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	NTS	NOT TO SCALE
ASM	ASSEMBLY (INTERNAL REFERENCE)	NO	NUMBER
ASTM	AMERICAN SOCIETY FOR TESTING AND MATLS	OC	ON CENTER
AWS	AMERICAN WELDING SOCIETY	OSHA	OCCUPATIONAL HEALTH AND SAFETY ADM.
CBC	CALIFORNIA BUILDING CODE	PCF	POUNDS PER CUBIC FOOT
CJP	COMPLETE JOINT PENETRATION	PD	POLYGON DRAWING
CLR	CLEAR	PJ	PRETENSIONED JOINT
DEG	DEGREE	PLCS	PLACES
DIA	DIAMETER	PLT	PLATE
DIM	DIMENSION	PSF	POUNDS PER SQUARE FOOT
DSA	DIVISION OF THE STATE ARCHITECT	PSI	POUNDS PER SQUARE INCH
EQ	EQUAL	QTY	QUANTITY
FT	FEET	REF	REFERENCE
GA	GAGE	SS	SQUARE
IN	INCHES	SQ	STANDING SEAM ROOF PANEL (MCELROY)
KSI	KIPS PER SQUARE INCH	TYP	TYPICAL
MAX	MAXIMUM	UNO	UNLESS NOTED OTHERWISE
MIN	MINIMUM	USGS	U.S. GEOLOGICAL SURVEY
MISC	MISCELLANEOUS	W/	WITH
MPH	MILES PER HOUR		

SPECIFICATIONS

PART 1 - GENERAL

- 1.1 STRUCTURE DESCRIPTION**
- A. STRUCTURE(S) BASED ON THE FOLLOWING PC DESIGN(S):
 - 1. MONOSLOPE (MSL)
- 1.2 DESIGN REQUIREMENTS**
- A. MEET THE DESIGN INTENT SHOWN ON THE PC DRAWINGS APPROVED FOR THIS PROJECT.
 - 1. DESIGN CRITERIA
 - 2. MEMBERS SIZES
 - 3. HIDDEN BOLTED CONNECTIONS BETWEEN STRUCTURAL MEMBERS
 - 4. COLUMN ANCHORAGE SHALL INCLUDE FOUR (4) BOLTS IN COMPLIANCE WITH OSHA 1926.755(A)(1).
 - 5. NO FIELD WELDING PERMITTED
 - 6. NO FIELD PAINTING PERMITTED
 - 7. ROOF DIMENSIONS AND SLOPES
 - 8. EXPOSED STEEL ROOF FASTENERS (IF APPLICABLE) POWDER COATED BY MANUFACTURER
 - 9. ROOF DECK SPANS FROM PEAK TO EAVE AND PERMITS PROPER DRAINAGE WITHOUT DEBRIS BUILD-UP.
- 1.3 SUBMITTALS**
- A. DRAWINGS AND CALCULATIONS SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE APPROPRIATE STATE.
 - B. ONLY MANUFACTURERS THAT SUBMIT DRAWINGS AND CALCULATIONS PRIOR TO BID SHALL BE CONSIDERED
 - C. MANUFACTURER MUST BE ABLE TO SUBMIT APPROPRIATE LABORATORY TESTS FOR THE FOLLOWING:
 - FRAME FINISH REQUIREMENTS LISTED IN PART 2 OF THIS SPECIFICATION.
 - CERTIFIED MILL TEST REPORTS FOR STRUCTURAL STEEL (DESCRIBING THE CHEMICAL AND PHYSICAL PROPERTIES)
 - CERTIFIED MILL TEST REPORTS FOR STRUCTURAL BOLTS.

- 1.4 TECHNICAL SUPPORT**
- A. MANUFACTURER MUST HAVE IN-HOUSE ENGINEERING DEPARTMENT AND A PROFESSIONAL ENGINEER LICENSED IN THE APPROPRIATE STATE TO ANSWER TECHNICAL QUESTIONS.

- 1.5 QUALITY ASSURANCE**
- A. GENERAL
 - FABRICATION PROCEDURES SHALL COMPLY WITH APPLICABLE CODES AND LOCAL REGULATIONS.
 - REQUIRED STRUCTURAL TESTS AND SPECIAL INSPECTIONS INCLUDED ON THE PROJECT DSA-103 FORM.
 - B. MANUFACTURER QUALIFICATIONS
 - MINIMUM (10) YEARS ENGINEERING AND FABRICATING PRE-ENGINEERED STRUCTURES
 - MANUFACTURER OWNED AND OPERATED POWDER COAT PAINT FINISH SYSTEM
 - ALL AWS CERTIFIED WELDERS
 - FULL-TIME PROFESSIONAL ENGINEER ON STAFF LICENSED IN THE APPROPRIATE STATE
 - FULL-TIME AWS CERTIFIED ASSOCIATE WELDING INSPECTOR ON STAFF
 - FULL-TIME QUALITY ASSURANCE MANAGER ON STAFF
 - FULL-TIME LEED AP ON STAFF
 - C. MANUFACTURER CERTIFICATIONS
 - PCI 4000 CERTIFICATION THROUGH POWDER COATING INSTITUTE (PCI)
 - AISC CERTIFIED FABRICATOR

- 1.6 MANUFACTURER WARRANTY**

GENERAL:

- 1. GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE THEY MAY CONFLICT WITH DETAILS AND NOTES ON OTHER SHEETS. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER FOR THIS PROJECT.
2. WORK SHALL CONFORM TO THE REQUIREMENTS, AS AMENDED TO DATE, OF THE LATEST ADOPTED EDITION OF THE CBC, C.A.C. TITLE 24, AND ALL OTHER LOCAL, STATE AND FEDERAL REGULATIONS.
3. OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT PRIOR TO PROCEEDING WITH ANY WORK INVOLVED.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS. ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT AND BE RESOLVED BEFORE PROCEEDING WITH THE WORK.
5. THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO, BRACING, TEMPORARY SUPPORTS, AND SHORING. OBSERVATION VISITS TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONSTRUCTION AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER, WHETHER OF MATERIAL OR WORK, ARE FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DOCUMENTS, BUT DO NOT GUARANTEE CONSTRUCTION.
6. ASTM DESIGNATIONS AND ALL STANDARDS REFER TO THE LATEST AMENDMENTS.
7. CONFORM TO APPLICABLE CAL/OSHA CONSTRUCTION SAFETY REGULATIONS FOR ALL WORK PERFORMED DURING CONSTRUCTION. JOB SITE SAFETY IS STRICTLY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ARCHITECT/ENGINEER OR OWNER.
8. THE ENGINEER AND THEIR CONSULTANTS SHALL HAVE NO RESPONSIBILITY FOR THE DISCOVERY, HANDLING, REMOVAL OR DISPOSAL OF HAZARDOUS MATERIALS AT THE PROJECT SITE, INCLUDING BUT NOT LIMITED, TO ASBESTOS, ASBESTOS PRODUCTS, POLYCHLORINATED BIPHENYL (PCB) OR OTHER TOXIC SUBSTANCES.
9. SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, OR IF A CHANGE IN THE SCOPE OF WORK IS PROPOSED, A CONSTRUCTION CHANGE DOCUMENT DETAILING AND SPECIFYING THE REQUIRED CHANGE(S) SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK.
10. THE SCHOOL DISTRICT'S INSPECTOR OF RECORD SHALL INSPECT AND APPROVE THE ERECTED FRAME PRIOR TO ROOF INSTALLATION.
11. SEE REQUIREMENTS FOR LOCATION IN ANY FIRE HAZARD SEVERITY ZONE FOR WILDLAND URBAN INTERFACE AREAS (WUI) AS SPECIFIED IN THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. PROVIDE PROTECTION AND DETAILS OF ALL AREAS COMPLYING WITH THE WUI REQUIREMENTS.
12. LOCATING THIS STRUCTURE CLOSER THAN 20 FEET TO OTHER STRUCTURES MAY AFFECT THE ALLOWABLE AREA FOR THE EXISTING CONSTRUCTION PER THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE.
13. VIEWS AND DETAILS ARE NOT DRAWN TO SCALE (UNLESS NOTED OTHERWISE). DO NOT SCALE THESE DRAWINGS.
14. OTHER SITE SPECIFIC ITEMS MAY BE REQUIRED.
15. WHEN A SITE-SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEER IS NEEDED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED IN THE PC ARE STILL APPLICABLE.

STRUCTURAL AND MISCELLANEOUS STEEL:

- 1. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERRECTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 360-16 AND 303-16 REFERENCED BY THE 2022 EDITION OF THE CALIFORNIA BUILDING CODE.
2. PIPE SECTIONS SHALL CONFORM TO ASTM A53, Fy = 35 ksi, GRADE B UNLESS NOTED OTHERWISE.
3. STRUCTURAL TUBING (HSS SHAPES) SHALL CONFORM TO ASTM A500, GRADE B (OR HIGHER), Fy = 46 KSI.
4. IF MATERIAL AVAILABILITY IS LIMITED, MEMBER THICKNESSES CAN BE INCREASED BEYOND WHAT IS SHOWN IN THESE DRAWINGS (MAXIMUM INCREASE OF 1/8").
5. ALL CHANNELS, ANGLES, PLATES AND MISC. STEEL SHALL CONFORM TO ASTM A36, Fy = 36 KSI.
6. ALL COLD FORM STEEL SHALL CONFORM TO ASTM A653, CS = TYPE B, Fy = 50 KSI.
7. STRUCTURAL STEEL AND DECK SHALL BE IDENTIFIED FOR CONFORMITY PER CBC 2202A.1.
8. ROOF DECK SHALL HAVE KYNAR 5000 METAL COATING.
9. ROOF DECK SHALL CONFORM TO ATSM A792, Fy = 50 KSI.
10. MR ROOF SCREWS MEET ASTM A510 WITH A HEAD DIMENSION OF 0.31" (FLAT-TO-FLAT) AND INTEGRAL WASHER DIMENSION OF 0.58" (OUTSIDE DIAMETER).
11. SS ROOF SCREWS MEET ASTM A510 WITH A HEAD DIMENSION OF 0.437" (OUTSIDE DIAMETER).

WELDING:

- 1. ALL WELDING SHALL COMPLY WITH AWS D1.1 SPECIFICATIONS AND SHALL BE DONE BY AWS QUALIFIED WELDERS CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED.
2. ALL WELDING SHALL BE DONE BY GAS METAL ARC PROCESS WITH E70XX ELECTRODES. FLUX CORE ARC WELD SHALL CONFORM TO CHARPY NOTCH TOUGHNESS RATING OF 20 ft-lb @ (0° F).
3. ALL WELDING SHALL BE DONE IN THE SHOP WITH REQUIRED INSPECTION, PRE-APPROVED BY DSA, TO ENSURE PROPER MATERIAL ID AND WELDING.
4. WELD FILLER METAL MANUFACTURER SHALL PROVIDE WRITTEN CERTIFICATION OF COMPLIANCE WITH CODE AND SPECIFICATIONS.

BOLTING:

- 1. ALL BOLTS SHOWN ON THESE DRAWINGS ARE ASTM F3125 (A325 TYPE 1) HIGH STRENGTH BOLTS (UNO) AND SHALL BE HOT DIPPED GALVANIZED PER ASTM F2329.
2. HIGH STRENGTH BOLTS SHALL BE SAMPLED AND TESTED IN COMPLIANCE WITH CBC 2213A.1.
3. BEFORE ERRECTING THE FRAME, VERIFY ALL BOLTS AND NUTS ARE CLEAN OF DEBRIS AND BURRS - INCLUDING THE HARDWARE ALREADY FASTENED INSIDE THE MEMBERS. CHECK SOME OF THE BOLTS AND NUTS MAY BE REQUIRED.
4. ANCHOR BOLTS (HEAVY HEX HEAD, ASTM F1554, GRADE 55) SHALL BE HOT DIPPED GALVANIZED PER ASTM F2329. ANCHOR BOLTS MAY BE HEADED OR THREADED WITH A NUT THAT IS PREVENTED FROM ROTATING.
5. HIGH STRENGTH NUTS SHALL CONFORM TO ASTM A563 AND SHALL BE GALVANIZED PER ASTM F2329.
6. HIGH STRENGTH WASHERS SHALL CONFORM TO ASTM F436 AND SHALL BE GALVANIZED PER ASTM F2329.
7. THE BOLTING INSTALLATION REQUIREMENTS OUTLINED BELOW ARE CRITICAL TO THE STRUCTURE'S DESIGN AND PERFORMANCE. THE INSTALLER IS REQUIRED TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME. ALL BOLTS SHALL BE INSTALLED AND INSPECTED PER THE APPLICABLE VERSION OF AISC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS", CBC 1705A.2.1; AISC 341-16 J7; AISC 360-16 N5.6.

- A. PRETENSIONED JOINTS (IDENTIFIED ON THE FRAME CONNECTION DETAILS WITH A "PJ REQUIRED") MUST BE INSTALLED AND INSPECTED TO MEET ONE OF FOLLOWING REQUIREMENTS:
1. TURN-OF-NUT PRETENSIONING
2. CALIBRATED WRENCH PRETENSIONING
3. DIRECT-TENSION-INDICATOR PRETENSIONING (CONTRACTOR RESPONSIBLE FOR PURCHASE OF REQUIRED WASHERS)
B. ALL OTHER JOINTS MUST BE INSTALLED AND INSPECTED TO MEET THE REQUIREMENTS OF SNUG-TIGHTENED JOINTS. NOTE TO INSTALLER AND INSPECTORS): THE SNUG-TIGHT CONDITION EXISTS, IN PART, WHEN ALL THE BOLTS IN THE JOINT HAVE BEEN TIGHTENED SUFFICIENTLY TO PREVENT THE REMOVAL OF THE NUTS WITHOUT THE USE OF A WRENCH.

THE CONTRACTOR, SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD MUST ALL AGREE ON WHICH APPROACH WILL BE USED TO PRETENSION THE BOLTS. THE CONTRACTOR IS RESPONSIBLE FOR DOCUMENTING THE APPROACH AGREED TO BY ALL PARTIES LISTED ABOVE.

FOUNDATIONS:

- 1. ALLOWABLE SOIL PRESSURES ASSUME CLASS 5 SOIL CLASSIFICATION PER 2022 CBC TABLE 1806A.2
2. FILL AND BACKFILL SHALL BE COMPACTED TO 95% OF MAX. DENSITY IN ACCORDANCE WITH ASTM TEST METHOD D1557. FLOODING NOT PERMITTED.
3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING, ETC. NECESSARY TO SUPPORT CUT AND/OR FILL BANKS DURING EXCAVATION, AND FORMING AND PLACEMENT OF CONCRETE.
4. STRUCTURES SHALL BE SETBACK FROM ADJACENT SLOPES TO PROVIDE FIRM MATERIAL FOR EMBEDMENT AND FOR PROTECTION FROM SLOPE DRAINAGE, EROSION, AND SHALLOW FAILURES.
A. BOTTOM OF ASCENDING SLOPE: THE SMALLER OF HALF THE HEIGHT OF THE SLOPE AND 15FT MEASURED FROM THE FACE OF THE STRUCTURE TO THE TOE OF THE SLOPE
B. TOP OF DESCENDING SLOPE: THE SMALLER OF A THIRD OF THE HEIGHT OF THE SLOPE AND 40 FT MEASURED FROM THE FACE OF THE FOOTING TO THE TOP OF THE SLOPE

ALTERNATE SETBACKS ARE PERMITTED, SUBJECT FOR APPROVAL. A GEOTECHNICAL INVESTIGATION MAY BE REQUIRED.

- 5. STRUCTURES PLACED ON LIQUIFIABLE SOILS OR SITE CLASS F MAY NOT BE SUBMITTED FOR AN OVER THE COUNTER REVIEW

CONCRETE:

- 1. MIX DESIGN REQUIREMENTS: (NORMAL WEIGHT CONCRETE)
MINIMUM STRENGTH f'c (28 DAYS) EXPOSURE CATEGORY MAXIMUM W/C RATIO SLUMP (± 1") UNIT WEIGHT (NORMAL WEIGHT)
5000 PSI F3, S3, W2, C2 0.4 4" 150 PCF
2. CHANGES TO THE MIX DESIGN MUST BE APPROVED BY THE ENGINEER OR ARCHITECT OF RECORD AND DSA
3. AGGREGATES SHALL CONFORM TO ASTM C33. MAX AGGREGATE SIZE = 1".
4. CEMENT SHALL CONFORM TO ASTM C150 (TYPE V) WITH A MAXIMUM EXPANSION OF 0.040%. FOR SULFATE RESISTANCE.
5. ADMIXTURES CONTAINING CALCIUM CHLORIDE ARE PROHIBITED.
6. CONCRETE EXPOSED TO FREEZING-AND-THAWING CYCLES SHALL BE AIR ENTRAINED PER ACI 318-19 SECTION 19.3.3.
7. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF FIVE DAYS AFTER PLACEMENT. ALTERNATE METHODS WILL BE APPROVED IF SATISFACTORY PERFORMANCE CAN BE ASSURED.
8. CONCRETE SHALL NOT FREE FALL MORE THAN FIVE FEET.
9. CONCRETE SHALL BE PROPORTIONED PER ACI 318-19 26.4.
10. CONCRETE SHALL BE TESTED PER CBC 1910A.1, 1705A.3, AND ACI 318-19 26.13. BATCH PLANT INSPECTION NOT REQUIRED. CONTRACTOR SHALL IMPLEMENT WEIGHTMASTER AND BATCH TICKET REQUIREMENTS OF CBC 1705A.3.3.1.

REINFORCING STEEL:

- 1. REINFORCING STEEL SHALL BE DEFORMED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A615. (DEFORMATIONS SHALL BE IN ACCORDANCE WITH ASTM A305) AS FOLLOWS:
GR 40: (#4 BARS AND LARGER)
GR 40: (#3 BARS)
2. DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS SHALL CONFORM TO THE ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCING CONCRETE STRUCTURES."
3. MIN. COVER FOR CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:
A. CAST AGAINST EARTH..... 3"
B. CAST AGAINST FORM BELOW GRADE..... 2"
C. FORMED SLABS (#11 BAR & SMALLER)..... 3/4"
D. SLABS ON GRADE (FROM TOP OF SLAB)..... 1"
E. COLUMNS AND BEAMS (MAIN BARS)..... 2"
F. WALLS EXPOSED TO WEATHER (#6-#18 BARS)..... 2" (#5 & SMALLER)..... 1 1/2"
G. NOT EXPOSED TO WEATHER (#11 & SMALLER)..... 3/4"
4. BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND. BENDS SHALL BE MADE COLD.
5. FOR #6 BARS AND SMALLER, REINFORCING SHALL BE LAP SPICED 45 BAR DIA MINIMUM IN CONCRETE. FOR #7 BARS AND LARGER, REINFORCING SHALL BE LAP SPICED 55 BAR DIAMETERS MINIMUM IN CONCRETE. ALL LAP SPICES MUST COMPLY WITH ACI 318-19.
6. PRIOR TO PLACING OF CONCRETE, REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE WELL SECURED IN POSITION.
7. WELDING OF REINFORCING IS NOT ALLOWED
8. REINFORCING STEEL SHALL BE SAMPLED AND TESTED PER CBC 1910A.2.

POWDER COATED AND EPOXY PRIMED FINISH:

- 1. ENTIRE POWDER COATING PROCESS COMPLETED IN SAME FACILITY AS STEEL FABRICATION.
2. ALL CARBON STEEL MEMBERS (COLUMNS, BEAMS, PLATES, ETC.) PAINTED WITH PRIME COAT PER THE "AISC CODE OF STANDARD PRACTICE" AND THE "AISC SPECIFICATION SECTION M3" (UNLESS NOTED OTHERWISE).
3. PARTS PRETREATED IN A 3 STAGE IRON PHOSPHATE WASHER (OR EQUAL).
4. EPOXY PRIMER POWDER COAT APPLIED TO PARTS FOR SUPERIOR CORROSION PROTECTION.
5. TOP POWDER COAT OF SUPER DURABLE TGIC (COLOR SELECTED FROM MANUFACTURER'S STANDARD OPTIONS OR CUSTOM COLOR).
6. SAMPLE PRODUCTION PARTS TESTED TO MEET THE FOLLOWING CRITERIA:
A. SALT SPRAY RESISTANCE PER ASTM B 117/ ASTM D 1654
1. 1000 HOURS WITH NO CREEP FROM SCRIBE LINE AND RATING OF 10
B. HUMIDITY RESISTANCE PER ASTM D2247-02
1. 5000 HOURS WITH NO LOSS OF ADHESION OR BLISTERING
C. COLOR/UV RESISTANCE PER ASTM G154-04
1. 2000 HOURS EXPOSURE ALTERNATE CYCLES WITH NO CHALKING, 75% COLOR RETENTION, AND COLOR VARIATION MAXIMUM 3.0 E VARIATION CIE FORMULA (BEFORE AND AFTER 2000 HOURS EXPOSURE

CONSTRUCTION NOTES

- 1. A DSA-CERTIFIED CLASS 2 INSPECTOR IS REQUIRED FOR THIS PROJECT.
2. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE DOCUMENT (CCD) APPROVED BY DSA, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24 CCR AND DSA IR A-6.
3. A "DSA-CERTIFIED" PROJECT INSPECTOR EMPLOYED BY THE OWNER (E.G. DISTRICT, ETC.) AND APPROVED BY DSA SHALL PROVIDE CONTINUOUS INSPECTION OF WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.
4. A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE OWNER (E.G. DISTRICT, ETC.) SHALL CONDUCT ALL THE REQUIRED TEST AND INSPECTIONS FOR THE PROJECT.

NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEER RESPONSIBILITY

- 1. FOR THE SITE-SPECIFIC PROJECT, NEITHER POLYGON OR GHD ARE THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE.
2. FOR THE SITE-SPECIFIC PROJECT, GHD AND POLYGON'S RESPONSIBILITY IS LIMITED TO THE PREPARATION OF THE PLANS AND SPECIFICATIONS FOR THE STRUCTURES OF THIS PC ONLY.
3. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM GHD AND POLYGON'S RESPONSIBILITY FOR THE SITE-SPECIFIC PROJECT.
4. ALL CONSTRUCTION ACTIVITIES RELATED TO STRUCTURAL ENGINEERING MAY BE DELEGATED TO A QUALIFIED ENGINEER BY THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE. THESE ACTIVITIES INCLUDE, BUT ARE NOT LIMITED TO, APPROVAL OF INSPECTOR QUALIFICATIONS, STRUCTURAL OBSERVATIONS OF CONSTRUCTION, REVIEW OF INSPECTIONS REPORTS, AND SIGNING OFF ON THE VERIFIED REPORT FOR COMPLETED WORK.
5. POLYGON WILL BE RESPONSIBLE FOR RESPONDING TO QUESTIONS PERTAINING TO THE PLANS AND SPECIFICATIONS FOR THE STRUCTURES OF THIS PC WHICH ARISE DURING PLAN REVIEW AND CONSTRUCTION.

SPECIAL INSPECTION NOTES:

- 1. THE PROJECT INSPECTOR AND TESTING AGENCY SHALL BE SELECTED BY THE SCHOOL DISTRICT AND APPROVED BY DSA AND THE ARCHITECT OF RECORD.
2. COSTS OF THE PROJECT INSPECTOR AND THE TESTING AGENCY SHALL BE BORN BY THE SCHOOL DISTRICT.
3. THE PROJECT INSPECTOR, AND ENTIRE CONSTRUCTION OVERSIGHT PROCESS, SHALL COMPLY WITH DSA PR 13-01.
4. ON APPROVED PC DRAWINGS, THE STATEMENT OF STRUCTURAL TESTS AND SPECIAL INSPECTIONS (FORM DSA-103) BELOW IS ONLY AN EXAMPLE. ON APPROVED PC DRAWINGS, THE EXAMPLE FORM DSA-103 MUST BE CROSSED OUT BEFORE THE PC DRAWINGS CAN BE APPROVED AS PART OF A SITE-SPECIFIC (OR STOCKPILE) PROJECT SO THEY WILL NOT CONFLICT WITH THE OFFICIAL FORM DSA-103 FOR THE PROJECT.

Table with 3 columns: Application Number, School Name, School District; DSA File Number, Increment Number, Date Submitted. Includes value 2022 CBC.

IMPORTANT: This form is only a summary list of structural tests and some of the special inspections required for the project. Generally, the structural tests and special inspections noted on this form are those that will be performed by the Geotechnical Engineer of Record, Laboratory of Record, or Special Inspector. The actual complete test and inspection program must be performed as detailed on the DSA approved documents. The appendix at the bottom of this form identifies work NOT subject to DSA requirements for special inspection or structural testing. The project inspector is responsible for providing inspection of all facets of construction, including but not limited to, special inspections not listed on this form such as structural wood framing, high-load wood diaphragms, cold-formed steel framing, anchorage of non-structural components, etc., per Title 24, Part 2, Chapter 17A (2022 CBC).

Table with 2 columns: 1. TYPE, 2. PERFORMED BY. Includes definitions for Continuous, Periodic, and Test inspections.

Table with 4 columns: S1. GENERAL, Test or Special Inspection, Type, Performed By, Code References and Notes. Includes S1.1 General and S1.2 Soil Compaction and Fill.

Table with 4 columns: S2. SOIL COMPACTION AND FILL, Test or Special Inspection, Type, Performed By, Code References and Notes. Includes S2.1 Soil Compaction and Fill.

Table with 4 columns: S4. CAST-IN-PLACE DEEP FOUNDATIONS (PIERS), Test or Special Inspection, Type, Performed By, Code References and Notes. Includes S4.1 Cast-in-place Deep Foundations (Piers).

Table with 4 columns: C1. CAST-IN-PLACE CONCRETE, Test or Special Inspection, Type, Performed By, Code References and Notes. Includes C1.1 Cast-in-place Concrete.

Table with 4 columns: S/A1. STRUCTURAL STEEL, COLD-FORMED STEEL AND ALUMINUM USED FOR STRUCTURAL PURPOSES, Test or Special Inspection, Type, Performed By, Code References and Notes. Includes S/A1.1 Structural Steel, Cold-formed Steel and Aluminum.

Table with 4 columns: S/A2. HIGH-STRENGTH BOLTS, Test or Special Inspection, Type, Performed By, Code References and Notes. Includes S/A2.1 High-strength Bolts.

Table with 4 columns: S/A3. WELDING, Test or Special Inspection, Type, Performed By, Code References and Notes. Includes S/A3.1 Welding.

Table with 4 columns: S/A4. SHOP WELDING (IN ADDITION TO SECTION S/A3), Test or Special Inspection, Type, Performed By, Code References and Notes. Includes S/A4.1 Shop Welding.

Table with 4 columns: S/A5. ANCHOR BOLTS AND ANCHOR RODS, Test or Special Inspection, Type, Performed By, Code References and Notes. Includes S/A5.1 Anchor Bolts and Anchor Rods.

Name of Architect or Engineer in general responsible charge:
Name of Structural Engineer (When structural design has been delegated):
Signature of Architect or Structural Engineer: Date:
Note: To facilitate DSA electronic mark-ups and identification stamp application, DSA recommends against using secured electronic or digital signatures.

Table with 2 columns: DSA STAMP, Signature/Date area.

- DSA 103-22: LIST OF REQUIRED VERIFIED REPORTS, CBC 2022
1. Soils Testing and Inspection: Geotechnical Verified Report Form DSA 293
2. Structural Testing and Inspection: Laboratory Verified Report Form DSA 291
3. Shop Welding Inspection: Laboratory Verified Report Form DSA 291, or, for independently contracting SI, Special Inspection Verified Report Form DSA 292
4. High-Strength Bolt Installation Inspection: Laboratory Verified Report Form DSA 291, or, for independently contracting SI, Special Inspection Verified Report Form DSA 292

4600 PLAZA GARDEN DRIVE
SUITE B
CAMERON PARK, CA 95002
530.677.5515



IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-121212 PC
REVIEWED FOR
SS [x] FLS [x] ACS [x] CG [x]
DATE: 7/18/2023

PRE-CHECK (PC) DOCUMENT
CODE: 2022 CBC
A SEPARATE PROJECT APPLICATION FOR CONSTRUCTION IS REQUIRED.

NOTES AND SPECIAL INSPECTIONS
MONOSLOPE -MSL

MSL1.1

FOUNDATION PLAN NOTES:

1. TOP OF ALL FOUNDATIONS MUST BE CONSTRUCTED AT ONE COMMON ELEVATION (COORDINATE WITH SITE PLANS - NOT BY POLYGON)
2. ALL FOUNDATIONS MUST BE CENTERED UNDER COLUMNS (UNO).
3. SEE SHEET MSL1.1 FOR CONCRETE REQUIREMENTS.
4. PRIOR TO FORMING AND CASTING FOUNDATIONS, REVIEW FOUNDATION PLAN FOR REQUIRED ORIENTATION.
5. FOUNDATION MATERIAL AND INSTALLATION NOT BY POLYGON.
6. VIBRATE CONCRETE FULL DEPTH OF FOUNDATION.

4090 PLAZA GOLDEN GATE CIRCLE
SUITE B
CAMERON PARK, CA 95002
ESD 677.3515

poligon
PORTER
A POLYGON COMPANY

REGISTERED PROFESSIONAL ENGINEER
JESSICA E. NAPIER
No. 55476
STRUCTURAL
STATE OF CALIFORNIA

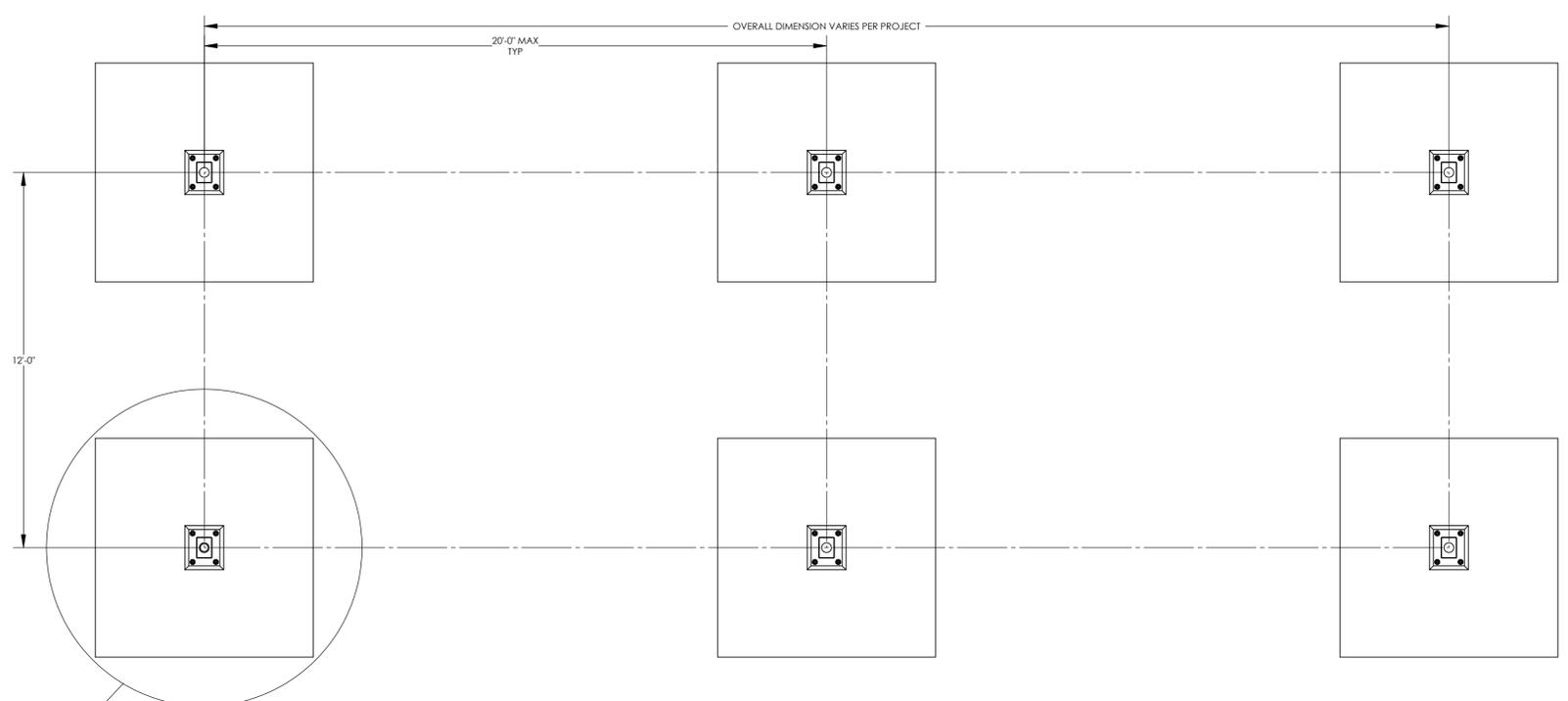
IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-121212 PC
REVIEWED FOR
SS FLS ACS CG
DATE: 7/18/2023

**PRE-CHECK (PC)
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A SEPARATE PROJECT
APPLICATION FOR
CONSTRUCTION IS REQUIRED.

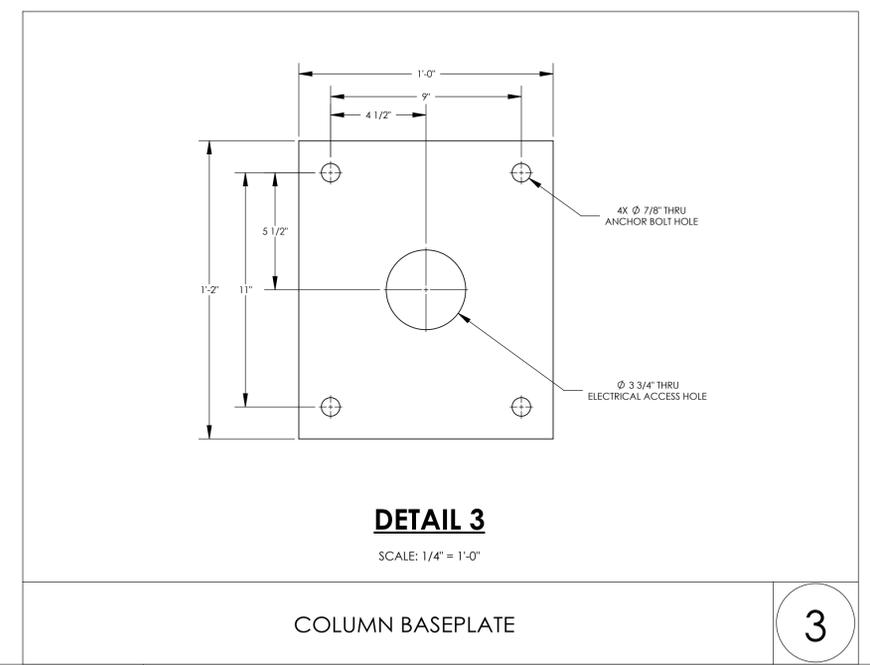
**FOUNDATION PLAN
SPREAD PAD**

MONOSLOPE - MSL 1.5

MSL2.0



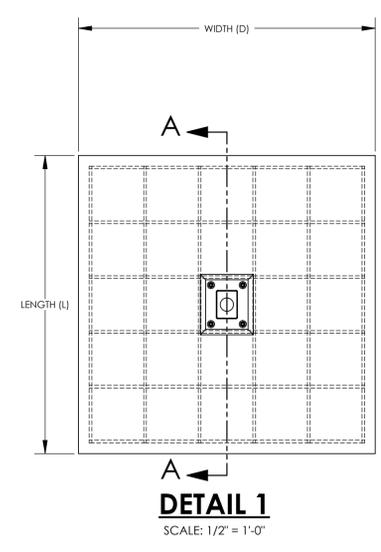
FOUNDATION PLAN (SPREAD PAD)
SCALE: 3/8" = 1'-0"



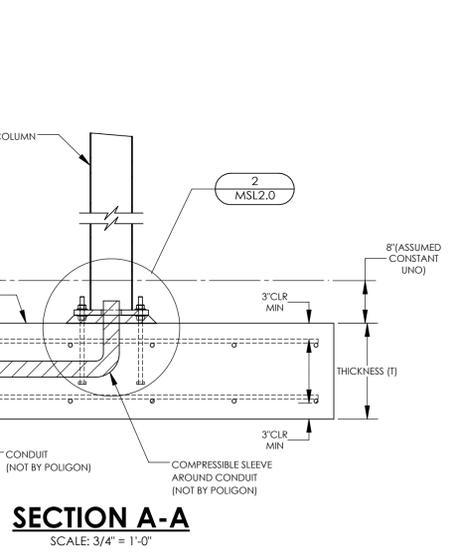
DETAIL 3
SCALE: 1/4" = 1'-0"

COLUMN BASEPLATE

3



DETAIL 1
SCALE: 1/2" = 1'-0"



SECTION A-A
SCALE: 3/4" = 1'-0"

SPREAD PAD FOUNDATION

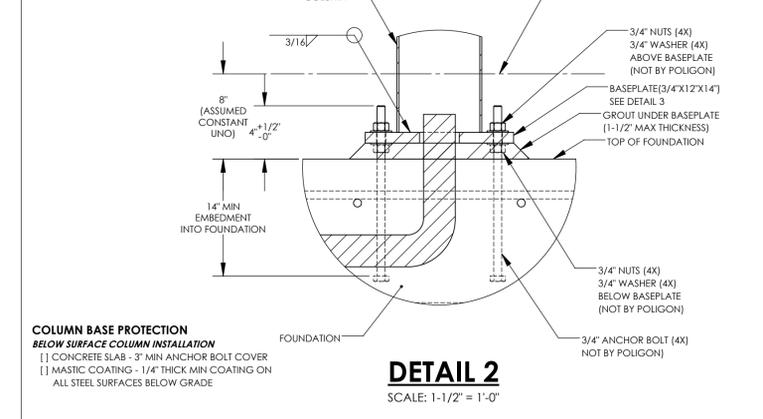
FOUNDATION REQUIREMENTS VARY PER PROJECT
SEE SHEET MSL1.0 FOR REQUIRED LOAD SCENARIO AND FOUNDATION TYPE (STEP 9 OF 'INSTRUCTIONS')
ONLY REFERENCE COPY OF PC DRAWINGS SUBMITTED FOR THIS PROJECT

LOAD SCENARIO	WIDTH (W)	LENGTH (L)	THICKNESS (T)	HORIZONTAL REINFORCING ¹	
				QTY	SIZE
1	7'-0"	7'-0"	1'-6"	7	#6
2	8'-0"	8'-0"	1'-6"	8	#6

¹ EQUALLY SPACED EACH WAY, TOP AND BOTTOM

NOTES:

1. GROUT SHALL BE NON-METALLIC, NON-SHRINK GROUT WITH MINIMUM f_c=6500 PSI.
2. COLUMNS FABRICATED ASSUMING 1-1/2" GROUT PAD.
3. COLUMN BASES BELOW FINISH GRADE SHOULD BE PROTECTED FROM CORROSION. SELECT ONE OF THE FOLLOWING OPTIONS BELOW.



DETAIL 2
SCALE: 1-1/2" = 1'-0"

COLUMN BASE PROTECTION
BELOW SURFACE COLUMN INSTALLATION
[] CONCRETE SLAB - 3" MIN ANCHOR BOLT COVER
[] MASTIC COATING - 1/4" THICK MIN COATING ON ALL STEEL SURFACES BELOW GRADE

COLUMN BASEPLATE AND ANCHOR BOLTS

1

2



PRE-CHECK (PC) DOCUMENT
 CODE: 2022 CBC
 A SEPARATE PROJECT APPLICATION FOR CONSTRUCTION IS REQUIRED.

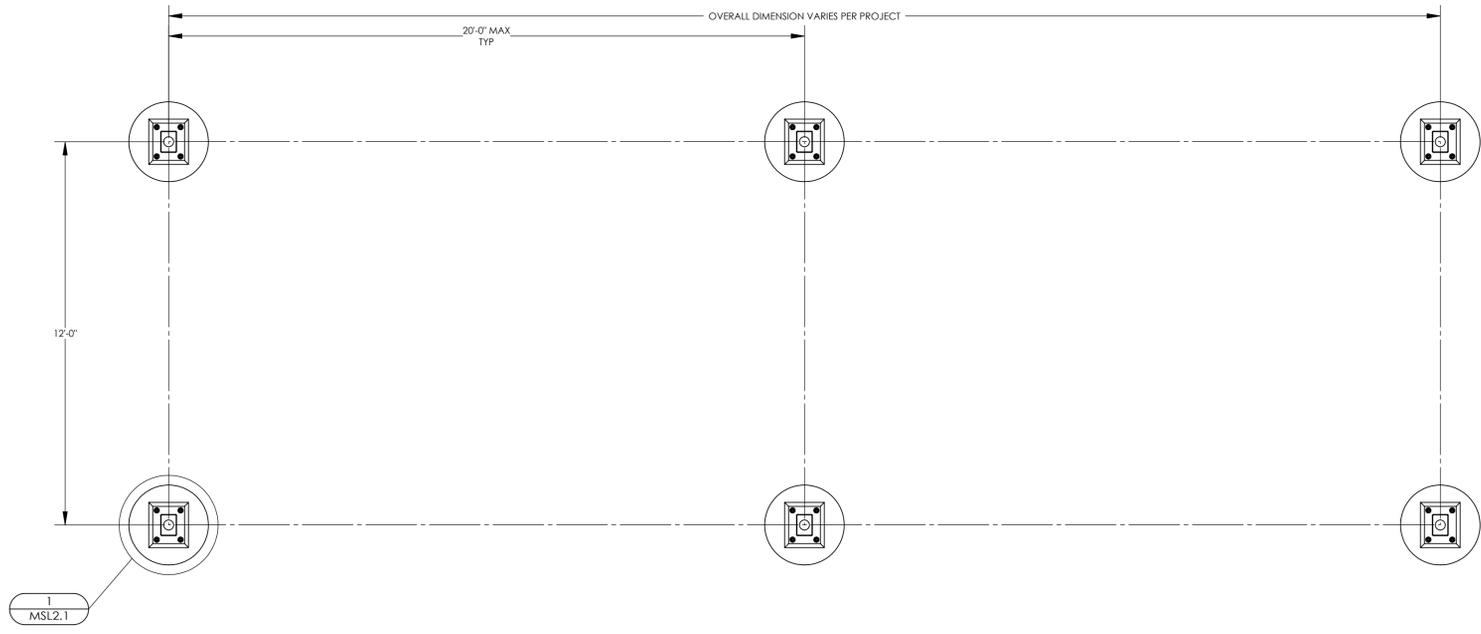
FOUNDATION PLAN DRILLED PIER

MONOSLOPE - MSL 1.5

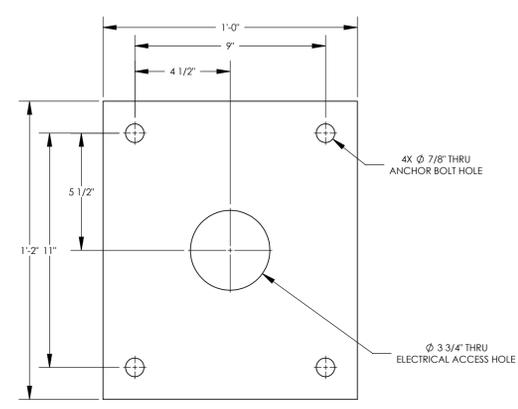
MSL2.1

FOUNDATION PLAN NOTES:

- TOP OF ALL FOUNDATIONS MUST BE CONSTRUCTED AT ONE COMMON ELEVATION (COORDINATE WITH SITE PLANS - NOT BY POLYGON)
- ALL FOUNDATIONS MUST BE CENTERED UNDER COLUMNS (UNO)
- SEE SHEET MSL1.1 FOR CONCRETE REQUIREMENTS.
- PRIOR TO FORMING AND CASTING FOUNDATIONS, REVIEW FOUNDATION PLAN FOR REQUIRED ORIENTATION.
- FOUNDATION MATERIAL AND INSTALLATION NOT BY POLYGON.
- VIBRATE CONCRETE FULL DEPTH OF FOUNDATION.
- FOR DRILLED PIER FOUNDATIONS, PREVENT SOIL FROM ENTERING EXCAVATED HOLE (FORM, ETC).



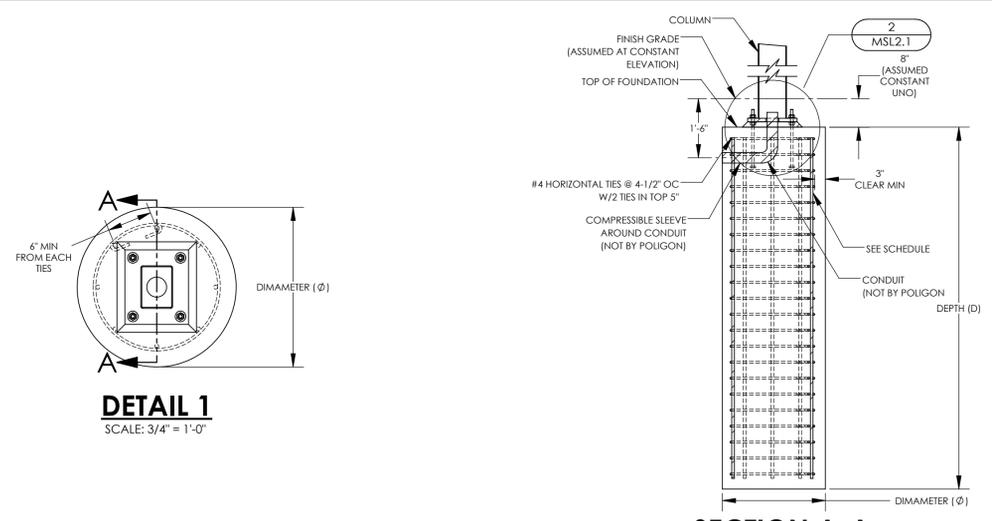
FOUNDATION PLAN (DRILLED PIER)
 SCALE: 3/8" = 1'-0"



DETAIL 3
 SCALE: 3" = 1'-0"

COLUMN BASEPLATE

3



SECTION A-A
 SCALE: 3/8" = 1'-0"

DRILLED PIER FOUNDATION

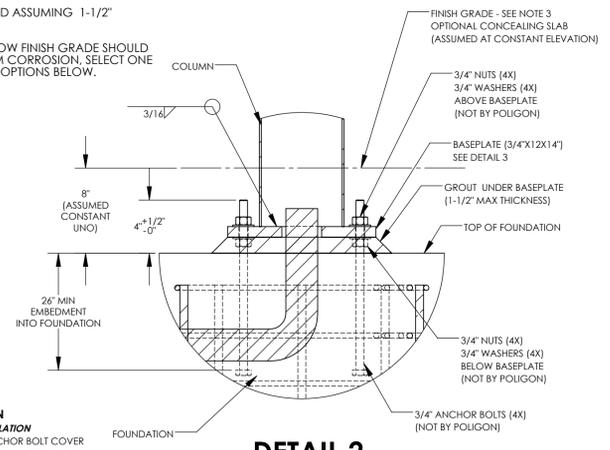
FOUNDATION REQUIREMENTS VARY PER PROJECT
 SEE SHEET MSL1.0 FOR REQUIRED LOAD SCENARIO AND FOUNDATION TYPE (STEP 9 OF 'INSTRUCTIONS')
 ONLY REFERENCE COPY OF PC DRAWINGS SUBMITTED FOR THIS PROJECT

LOAD SCENARIO	DIAMETER (Ø)	DEPTH (D)	VERTICAL REINFORCING ¹	
			QTY	SIZE
1	2'-6"	10'-6"	8	#6
2	2'-6"	11'-0"	8	#6

¹ EQUALLY SPACED AROUND DRILLED PIER

NOTES:

- GROUT SHALL BE NON-METALLIC, NON-SHRINK GROUT WITH MINIMUM $f_c=6500$ PSL.
- COLUMNS FABRICATED ASSUMING 1-1/2" GROUT PAD.
- COLUMN BASES BELOW FINISH GRADE SHOULD BE PROTECTED FROM CORROSION; SELECT ONE OF THE FOLLOWING OPTIONS BELOW.



DETAIL 2
 SCALE: 1-1/2" = 1'-0"

COLUMN BASEPLATE AND ANCHOR BOLTS

1

2

FOUNDATION PLAN NOTES:

1. TOP OF ALL FOUNDATIONS MUST BE CONSTRUCTED AT ONE COMMON ELEVATION (COORDINATE WITH SITE PLANS - NOT BY POLYGON)
2. ALL FOUNDATIONS MUST BE CENTERED UNDER COLUMNS (UNO).
3. SEE SHEET MSL1.1 FOR CONCRETE REQUIREMENTS.
4. PRIOR TO FORMING AND CASTING FOUNDATIONS, REVIEW FOUNDATION PLAN FOR REQUIRED ORIENTATION.
5. FOUNDATION MATERIAL AND INSTALLATION NOT BY POLYGON.
6. VIBRATE CONCRETE FULL DEPTH OF FOUNDATION.

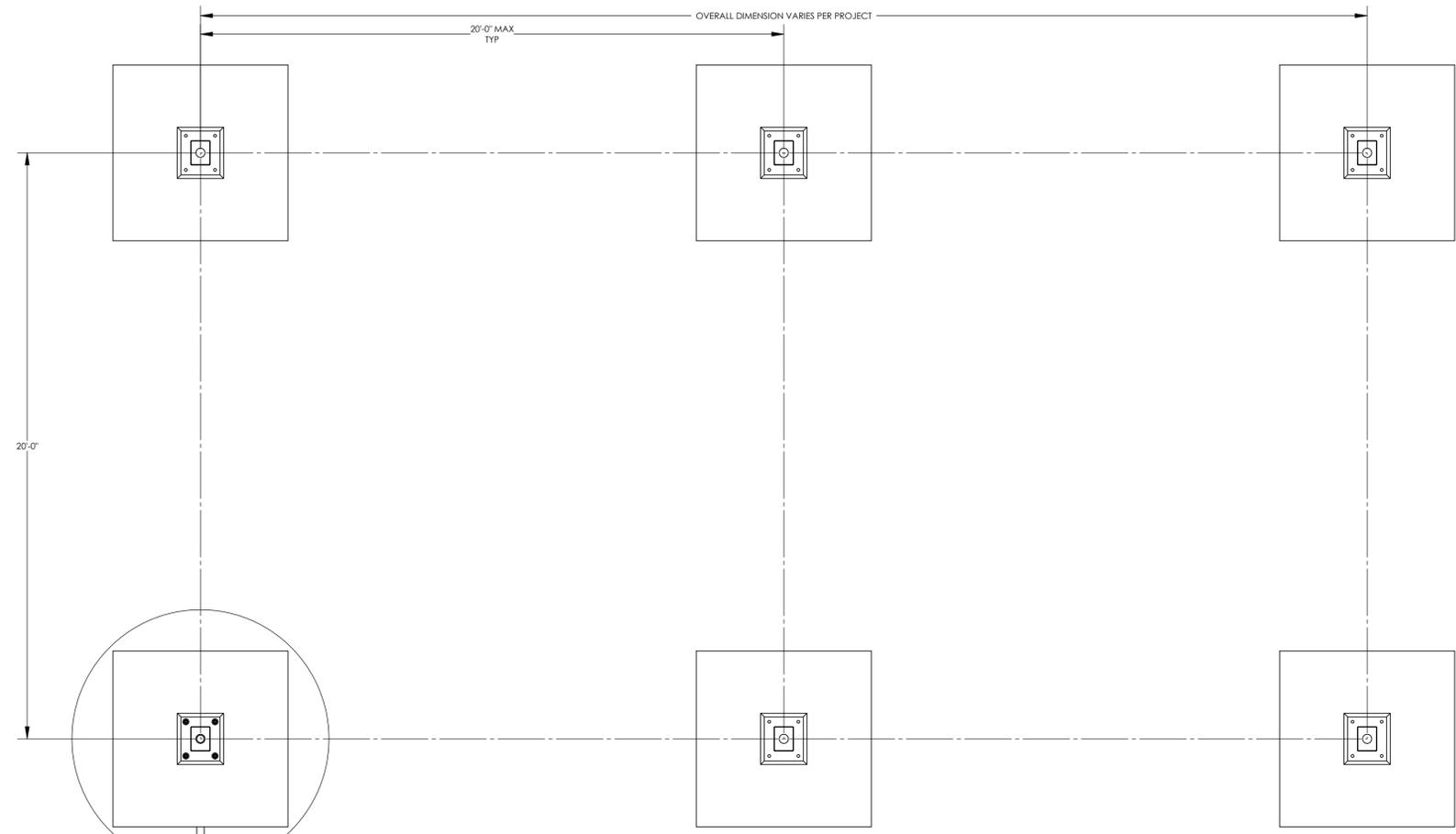


PRE-CHECK (PC) DOCUMENT
 CODE: 2022 CBC
 A SEPARATE PROJECT APPLICATION FOR CONSTRUCTION IS REQUIRED.

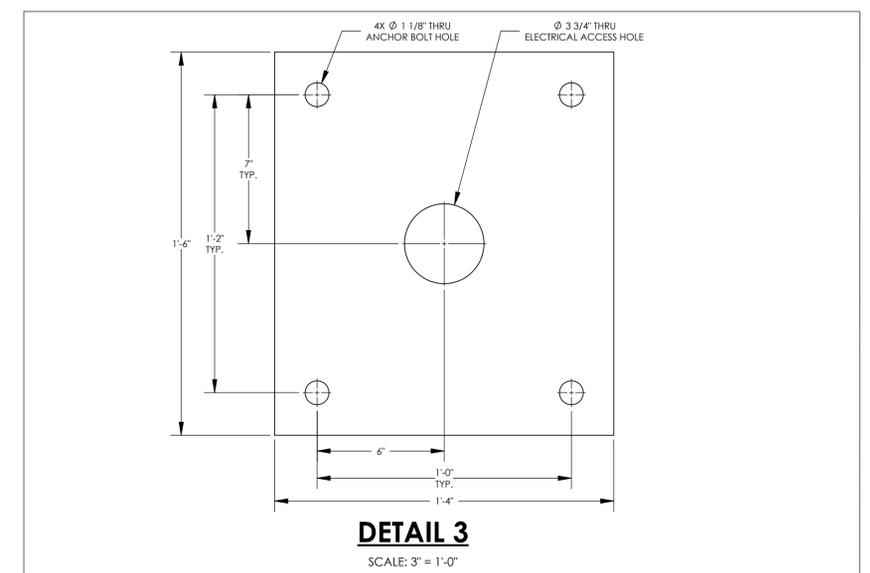
FOUNDATION PLAN SPREAD PAD

MONOSLOPE - MSL 2.5

MSL2.2



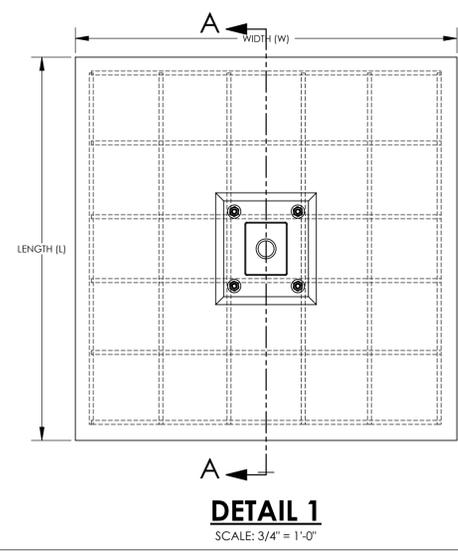
FOUNDATION PLAN (SPREAD PAD)
 SCALE: 3/8" = 1'-0"



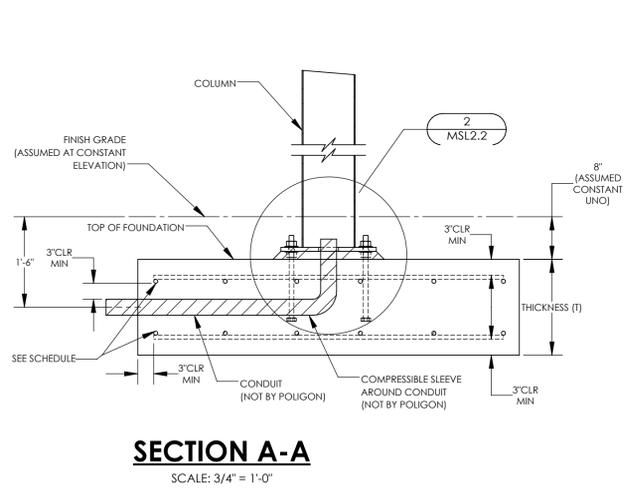
DETAIL 3
 SCALE: 3" = 1'-0"

COLUMN BASEPLATE

3



DETAIL 1
 SCALE: 3/4" = 1'-0"



SECTION A-A
 SCALE: 3/4" = 1'-0"

SPREAD PAD FOUNDATION

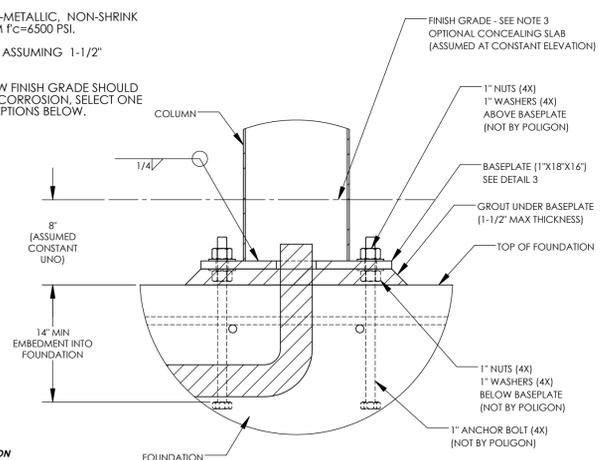
FOUNDATION REQUIREMENTS VARY PER PROJECT
 SEE SHEET MSL1.0 FOR REQUIRED LOAD SCENARIO AND FOUNDATION TYPE (STEP 9 OF 'INSTRUCTIONS')
 ONLY REFERENCE COPY OF PC DRAWINGS SUBMITTED FOR THIS PROJECT

LOAD SCENARIO	WIDTH (W)	LENGTH (L)	THICKNESS (T)	HORIZONTAL REINFORCING ¹	
				QTY	SIZE
1	8'-6"	8'-6"	1'-6"	8	#6
2	9'-6"	9'-6"	1'-6"	9	#6

¹EQUALLY SPACED EACH WAY, TOP AND BOTTOM

NOTES:

1. GROUT SHALL BE NON-METALLIC, NON-SHRINK GROUT WITH MINIMUM $f_c=6500$ PSI.
2. COLUMNS FABRICATED ASSUMING 1-1/2" GROUT PAD.
3. COLUMN BASES BELOW FINISH GRADE SHOULD BE PROTECTED FROM CORROSION, SELECT ONE OF THE FOLLOWING OPTIONS BELOW.



DETAIL 2
 SCALE: 1-1/2" = 1'-0"

COLUMN BASE PROTECTION
 BELOW SURFACE COLUMN INSTALLATION
 [] CONCRETE SLAB - 3" MIN ANCHOR BOLT COVER
 [] MASTIC COATING - 1/4" THICK MIN COATING ON ALL STEEL SURFACES BELOW GRADE

COLUMN BASEPLATE AND ANCHOR BOLTS

2

1

FOUNDATION PLAN NOTES:

1. TOP OF ALL FOUNDATIONS MUST BE CONSTRUCTED AT ONE COMMON ELEVATION (COORDINATE WITH SITE PLANS - NOT BY POLYGON)
2. ALL FOUNDATIONS MUST BE CENTERED UNDER COLUMNS (UNO).
3. SEE SHEET MSL1.1 FOR CONCRETE REQUIREMENTS.
4. PRIOR TO FORMING AND CASTING FOUNDATIONS, REVIEW FOUNDATION PLAN FOR REQUIRED ORIENTATION.
5. FOUNDATION MATERIAL AND INSTALLATION NOT BY POLYGON.
6. VIBRATE CONCRETE FULL DEPTH OF FOUNDATION.
7. FOR DRILLED PIER FOUNDATIONS, PREVENT SOIL FROM ENTERING EXCAVATED HOLE (FORM, ETC).

4099 PLAZA GOLDEN GATE CIRCLE
SUITE B
CAMERON PARK, CA 95002
ESD:677.3515



poligon
PORTER
A POLYCON COMPANY



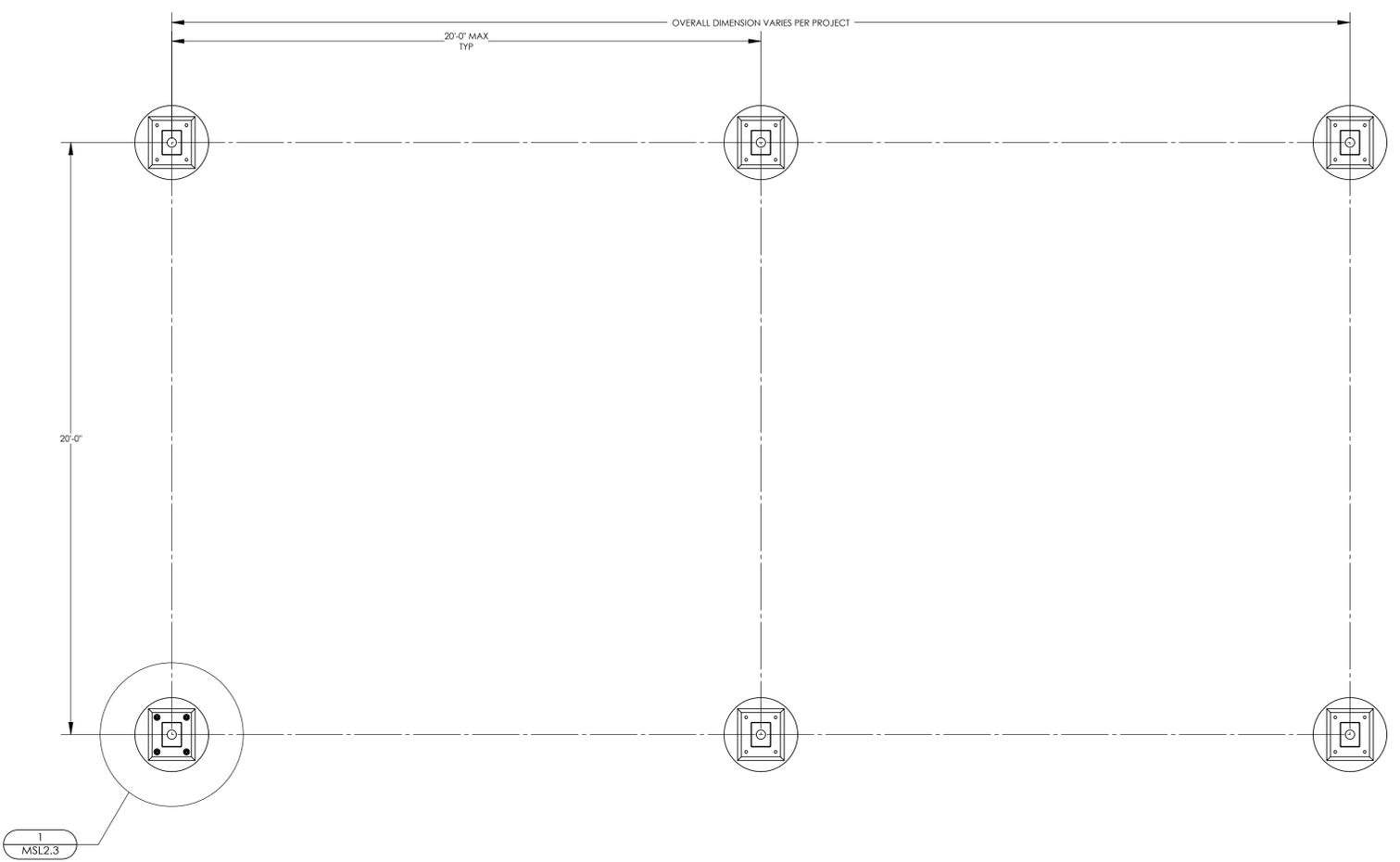
IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-121212 PC
REVIEWED FOR
SS FLS ACS CG
DATE: 7/18/2023

**PRE-CHECK (PC)
DOCUMENT
CODE: 2022 CBC
A SEPARATE PROJECT
APPLICATION FOR
CONSTRUCTION IS REQUIRED.**

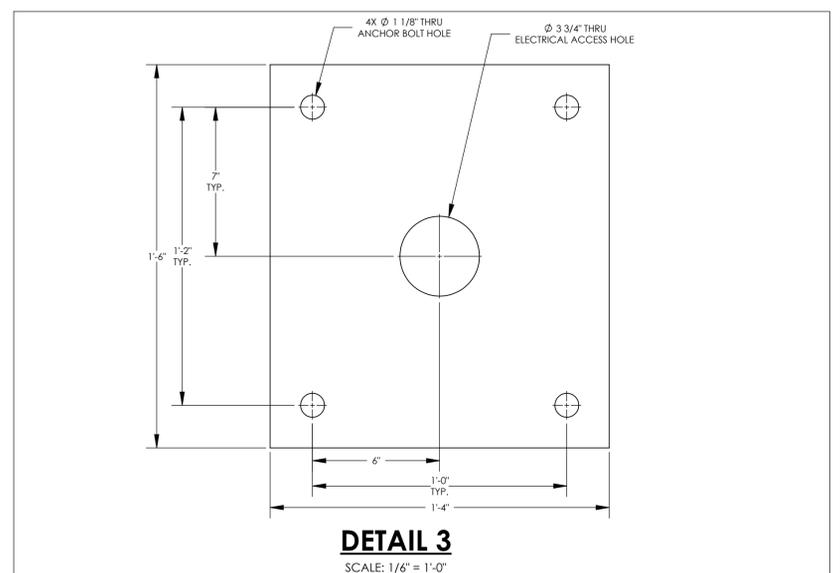
**FOUNDATION PLAN
DRILLED PIER**

MONOSLOPE - MSL 2.5

MSL2.3

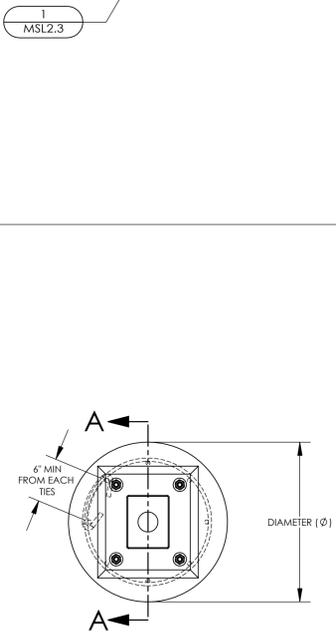


FOUNDATION PLAN (DRILLED PIER)
SCALE: 3/8" = 1'-0"

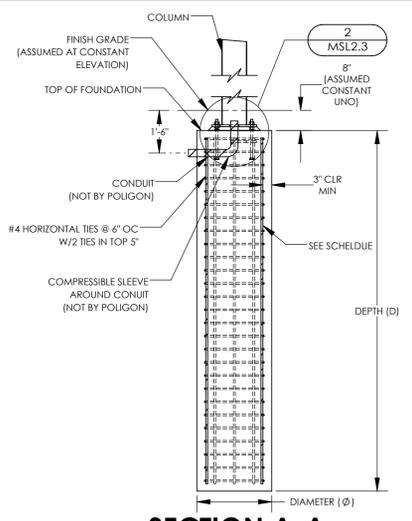


DETAIL 3
SCALE: 1/4" = 1'-0"

3



DETAIL 1
SCALE: 3/4" = 1'-0"



SECTION A-A
SCALE: 3/8" = 1'-0"

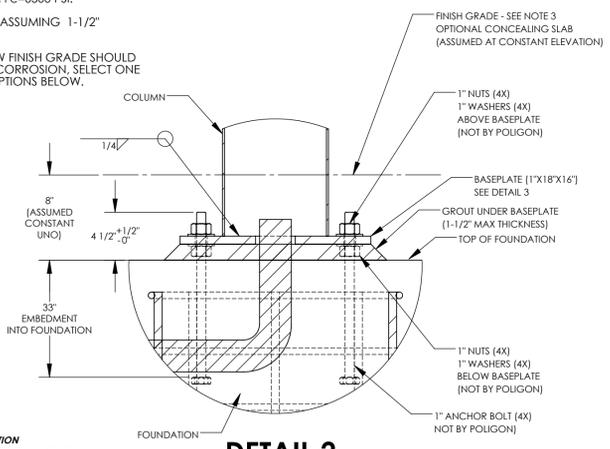
FOUNDATION REQUIREMENTS VARY PER PROJECT
SEE SHEET MSL1.0 FOR REQUIRED LOAD SCENARIO AND FOUNDATION TYPE (STEP 9 OF 'INSTRUCTIONS')
ONLY REFERENCE COPY OF PC DRAWINGS SUBMITTED FOR THIS PROJECT

LOAD SCENARIO	DIAMETER (Ø)	DEPTH (D)	VERTICAL REINFORCING ¹	
			QTY	SIZE
1	2'-6"	12'-6"	8	#8
2	2'-6"	14'-0"	8	#8

¹ EQUALLY SPACED AROUND DRILLED PIER

NOTES:

1. GROUT SHALL BE NON-METALLIC, NON-SHRINK GROUT WITH MINIMUM Fc=6500 PSI.
2. COLUMNS FABRICATED ASSUMING 1-1/2" GROUT PAD.
3. COLUMN BASES BELOW FINISH GRADE SHOULD BE PROTECTED FROM CORROSION. SELECT ONE OF THE FOLLOWING OPTIONS BELOW.



**COLUMN BASE PROTECTION
BELOW SURFACE COLUMN INSTALLATION**
[1] CONCRETE SLAB - 5" MIN ANCHOR BOLT COVER
[1] MASTIC COATING - 1/4" THICK MIN COATING ON ALL STEEL SURFACES BELOW GRADE

DETAIL 2
SCALE: 1-1/2" = 1'-0"

1

2

FOUNDATION PLAN NOTES:

1. TOP OF ALL FOUNDATIONS MUST BE CONSTRUCTED AT ONE COMMON ELEVATION (COORDINATE WITH SITE PLANS - NOT BY POLYGON)
2. ALL FOUNDATIONS MUST BE CENTERED UNDER COLUMNS (UNO).
3. SEE SHEET MSL1.1 FOR CONCRETE REQUIREMENTS.
4. PRIOR TO FORMING AND CASTING FOUNDATIONS, REVIEW FOUNDATION PLAN FOR REQUIRED ORIENTATION.
5. FOUNDATION MATERIAL AND INSTALLATION NOT BY POLYGON.
6. VIBRATE CONCRETE FULL DEPTH OF FOUNDATION.

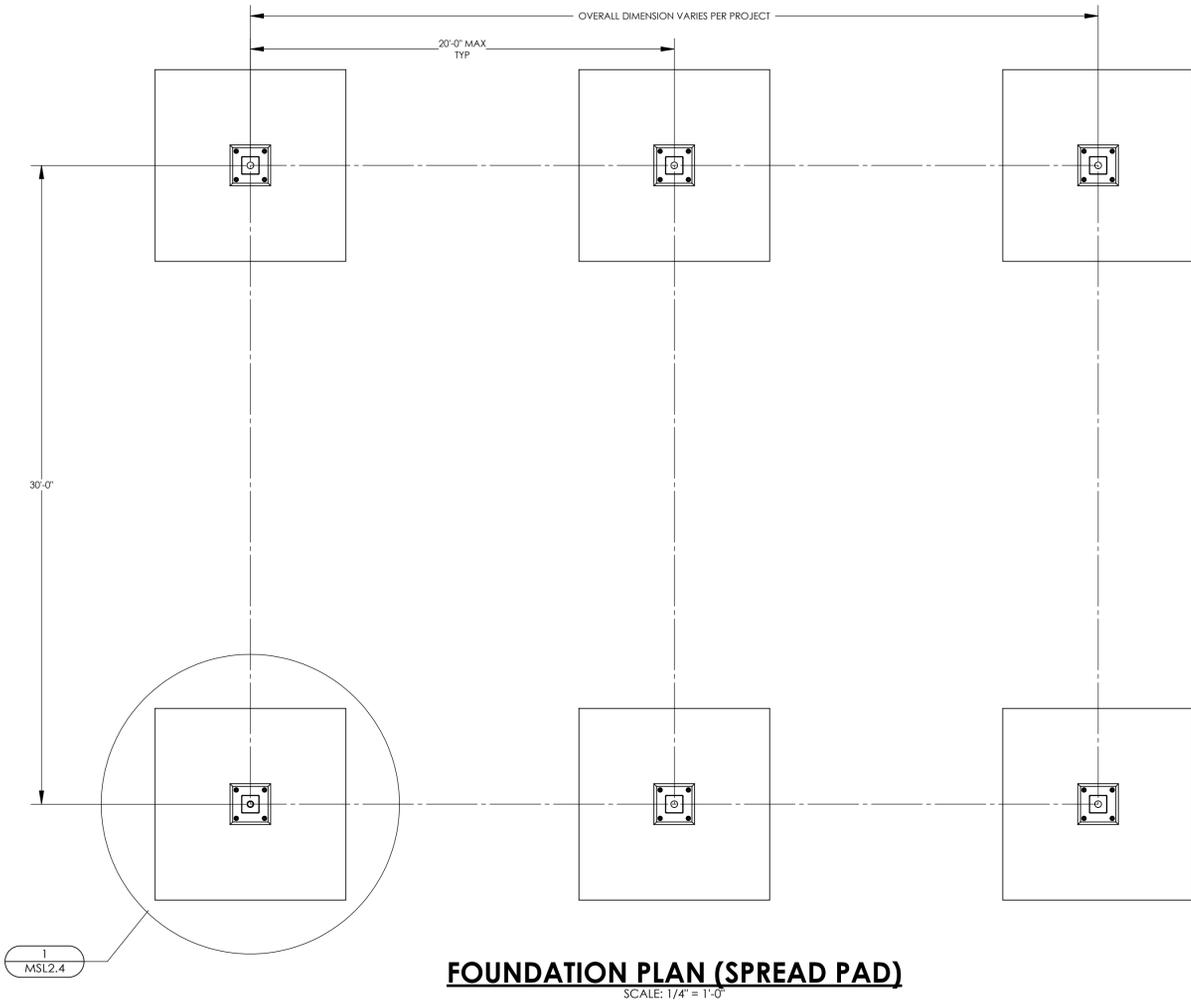


PRE-CHECK (PC) DOCUMENT
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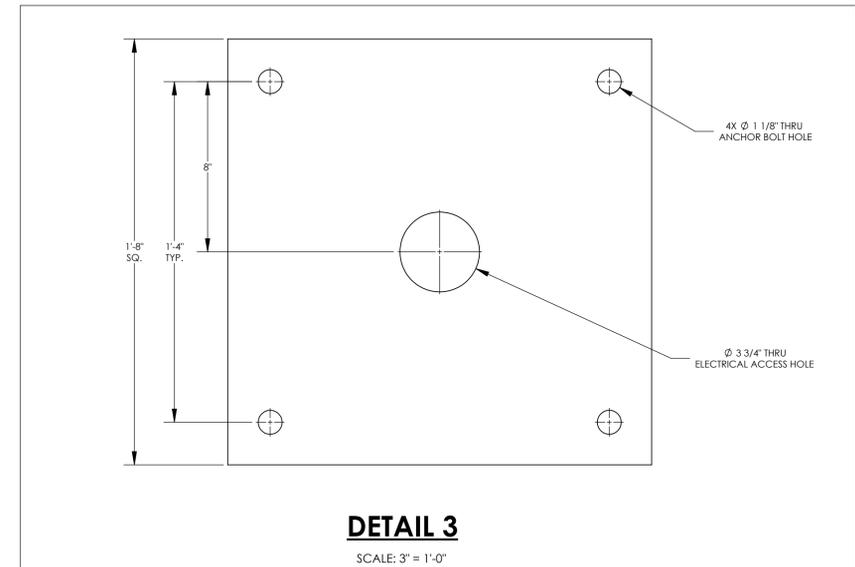
FOUNDATION PLAN SPREAD PAD

MONOSLOPE - MSL 35

MSL2.4



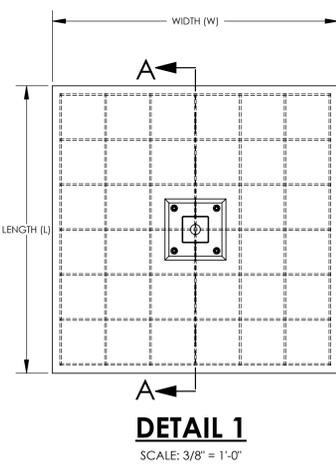
FOUNDATION PLAN (SPREAD PAD)
 SCALE: 1/4" = 1'-0"



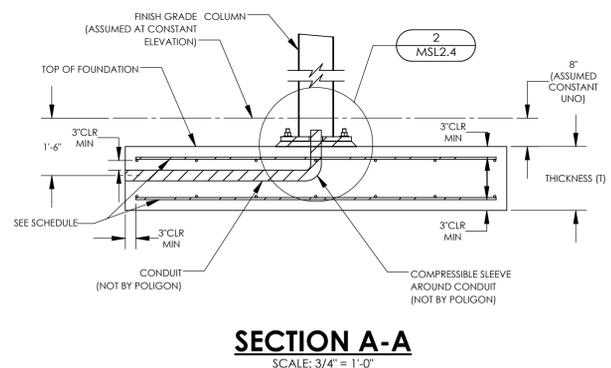
DETAIL 3
 SCALE: 3" = 1'-0"

COLUMN BASEPLATE

3



DETAIL 1
 SCALE: 3/8" = 1'-0"



SECTION A-A
 SCALE: 3/4" = 1'-0"

SPREAD PAD FOUNDATION

FOUNDATION REQUIREMENTS VARY PER PROJECT
SEE SHEET MSL1.0 FOR REQUIRED LOAD SCENARIO AND FOUNDATION TYPE (STEP 9 OF 'INSTRUCTIONS')
 ONLY REFERENCE COPY OF PC DRAWINGS SUBMITTED FOR THIS PROJECT

LOAD SCENARIO	WIDTH (W)	LENGTH (L)	THICKNESS (T)	HORIZONTAL REINFORCING ¹	
				QTY	SIZE
1	10'-0"	10'-0"	1'-6"	9	#6
2	11'-6"	11'-6"	1'-6"	11	#6

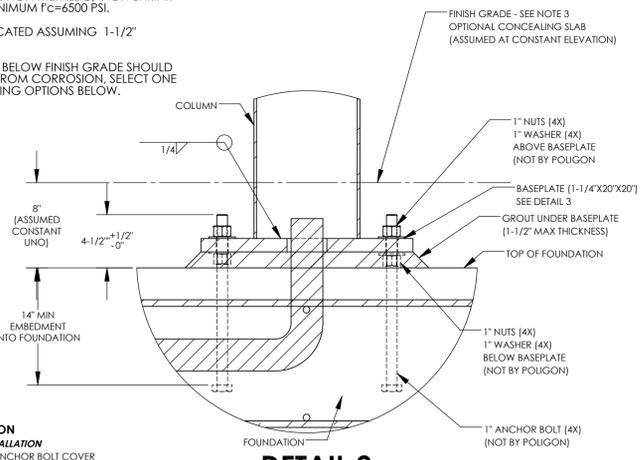
¹ EQUALLY SPACED EACH WAY, TOP AND BOTTOM

NOTES:

1. GROUT SHALL BE NON-METALLIC, NON-SHRINK GROUT WITH MINIMUM f_c=6500 PSI.
2. COLUMNS FABRICATED ASSUMING 1-1/2" GROUT PAD.
3. COLUMN BASES BELOW FINISH GRADE SHOULD BE PROTECTED FROM CORROSION, SELECT ONE OF THE FOLLOWING OPTIONS BELOW.

COLUMN BASE PROTECTION

- [1] CONCRETE SLAB - 3" MIN ANCHOR BOLT COVER
- [1] MASTIC COATINGS - 1/4" THICK MIN COATING ON ALL STEEL SURFACES BELOW GRADE



DETAIL 2
 SCALE: 1-1/2" = 1'-0"

COLUMN BASEPLATE AND ANCHOR BOLTS

2

1

STATE APPROVALS-SITE

4090 PLAZA GOLDEN GATE CIRCLE
SUITE B
CAMERON PARK, CA 95002
530.677.3515

poligon PORTER
A POLYGON COMPANY

REGISTERED PROFESSIONAL ARCHITECT
JESSICA E. MAPER
No. 55476
STRUCTURAL
STATE OF CALIFORNIA

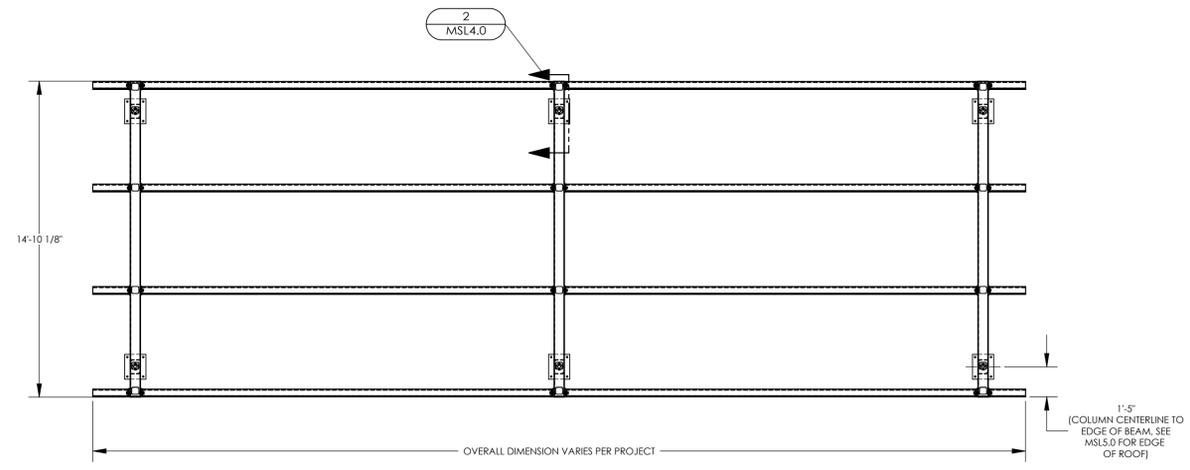
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IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-121212 PC
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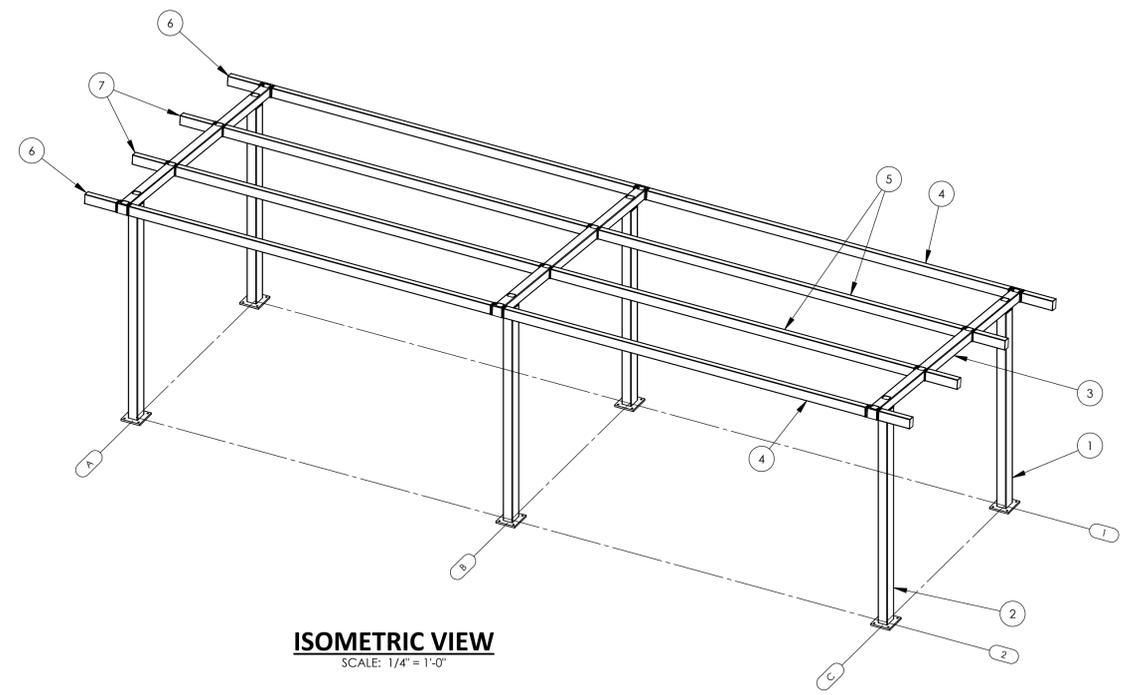
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CODE: 2022 CBC
A SEPARATE PROJECT
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FRAMING PLAN
MONOSLOPE - MSL 1.5

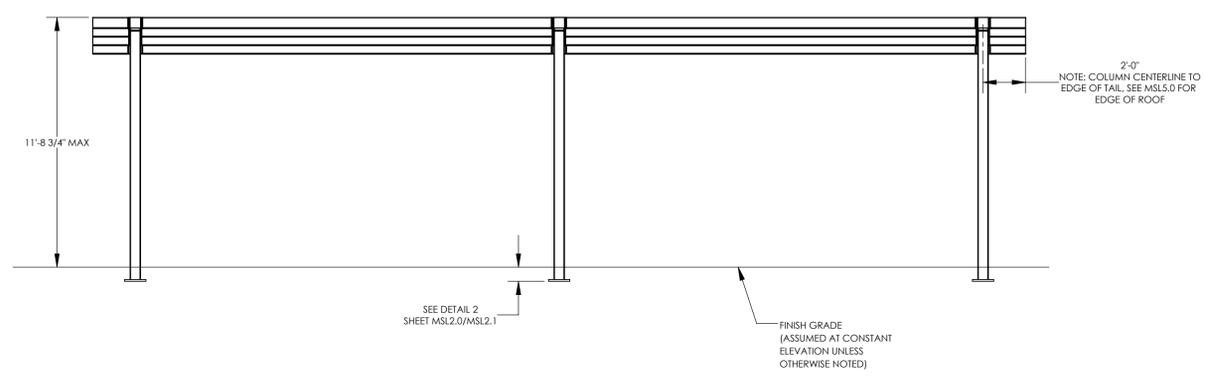
MSL3.0



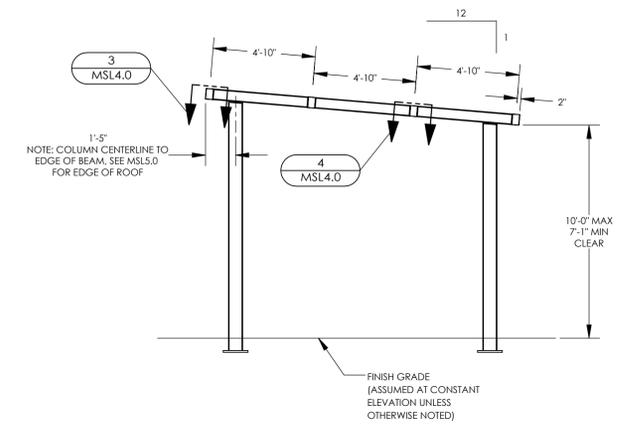
PLAN VIEW
SCALE: 1/4" = 1'-0"



ISOMETRIC VIEW
SCALE: 1/4" = 1'-0"



FRONT ELEVATION
SCALE: 1/4" = 1'-0"



SIDE ELEVATION
SCALE: 1/4" = 1'-0"

ITEM	QTY.	PART NO.	DESCRIPTION	MATERIAL	WEIGHT
7	4	-	PURLIN TAIL ASM	HSS6x4x1/8	16.92
6	4	-	EBEAM TAIL ASM	HSS6x4x1/8	18.13
5	4	-	PURLIN ASM	HSS6x4x1/8	161.76
4	4	-	EAVE BEAM ASM	HSS6x4x1/8	164.19
3	3	-	GABLE BEAM_1 ASM	HSS6X6X3/16	249.59
2	3	-	COL_2 ASM	HSS8X6X3/16	245.60
1	3	-	COL_1 ASM	HSS8X6X3/16	228.63

STATE APPROVALS-SITE

4099 PLAZA GOLDEN GATE CIRCLE
SUITE B
CAMERON PARK, CA 95002
530.677.3515

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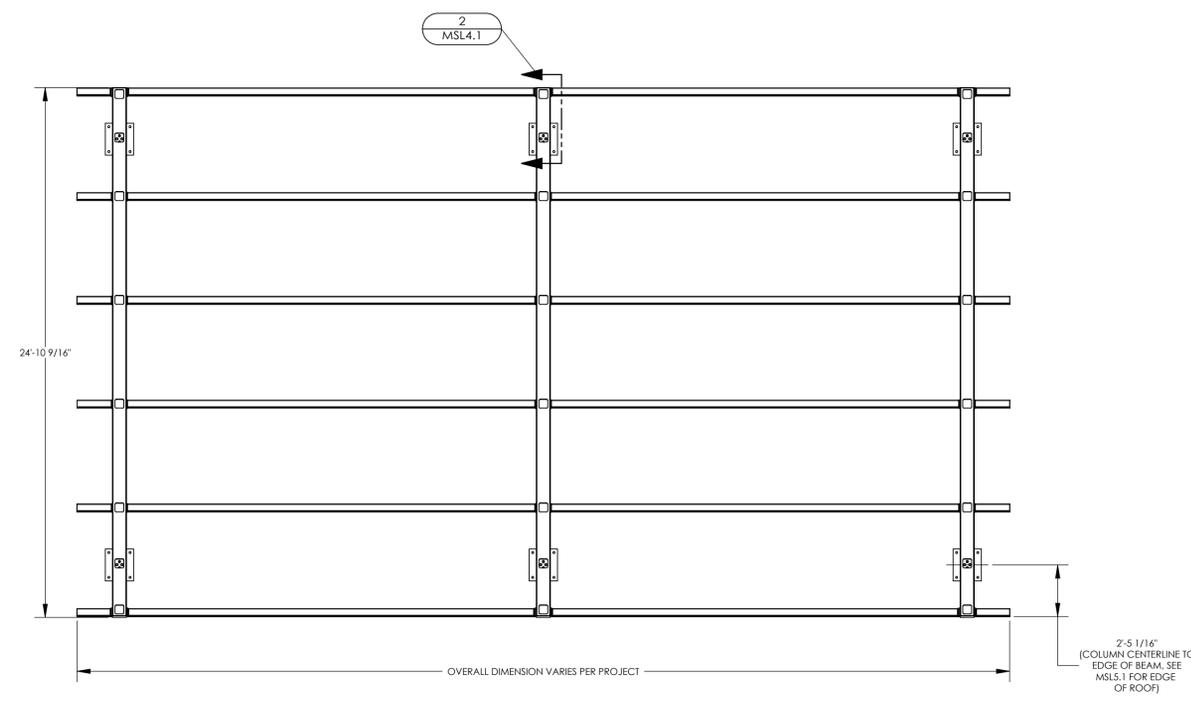
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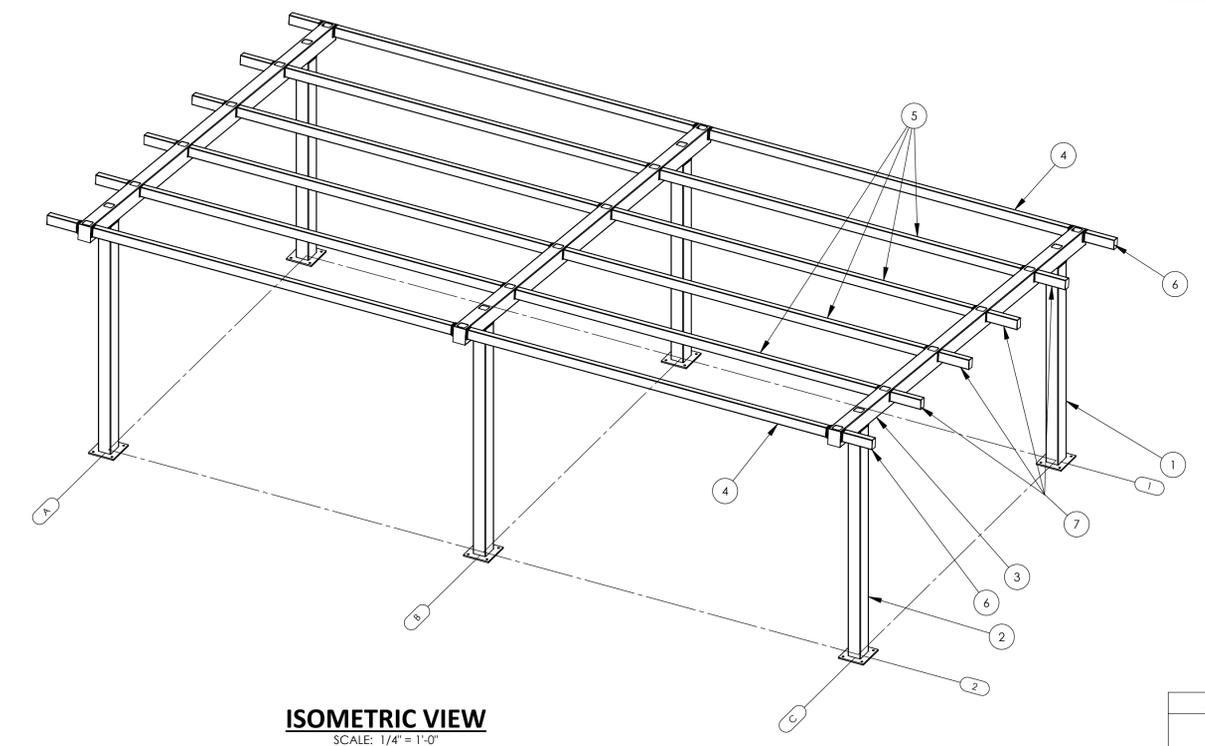
PRE-CHECK (PC)
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CODE: 2022 CBC
A SEPARATE PROJECT
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FRAMING PLAN
MONOSLOPE - MSL 25

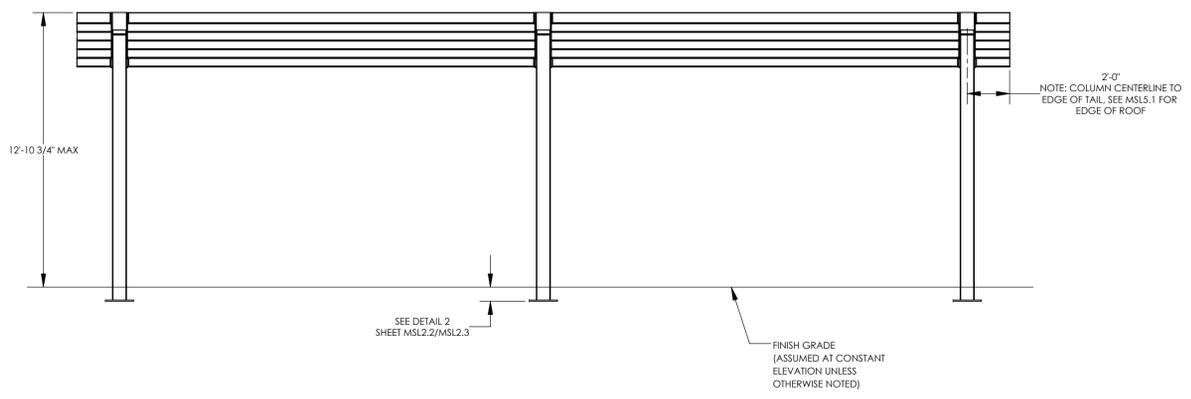
MSL3.1



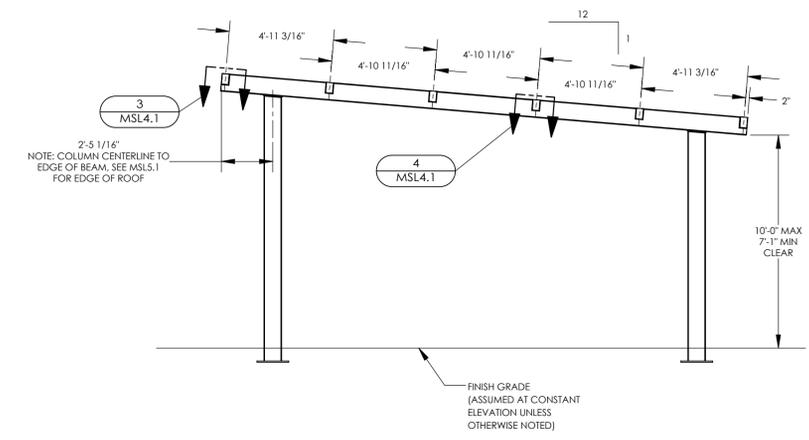
PLAN VIEW
SCALE: 1/4" = 1'-0"



ISOMETRIC VIEW
SCALE: 1/4" = 1'-0"



FRONT ELEVATION
SCALE: 1/4" = 1'-0"



SIDE ELEVATION
SCALE: 1/4" = 1'-0"

ITEM	QTY.	PART NO.	DESCRIPTION	MATERIAL	WEIGHT
7	8	-	PURLIN TAIL ASM	HSS6X4X1/8	16.39
6	4	-	EBEAM TAIL ASM	HSS6X4X1/8	17.51
5	8	-	PURLIN ASM	HSS6X4X1/8	160.58
4	4	-	EAVE BEAM ASM	HSS6X4X1/8	162.84
3	3	-	GABLE_1 ASM	HSS10X8X3/16	594.19
2	3	-	COL_2 ASM	HSS10X8X3/16	339.19
1	3	-	COL_1 ASM	HSS10X8X3/16	302.45

STATE APPROVALS-SITE

4090 PLAZA GOLDEN GATE CIRCLE
SUITE B
CAMERON PARK, CA 95002
ESD:677.3515

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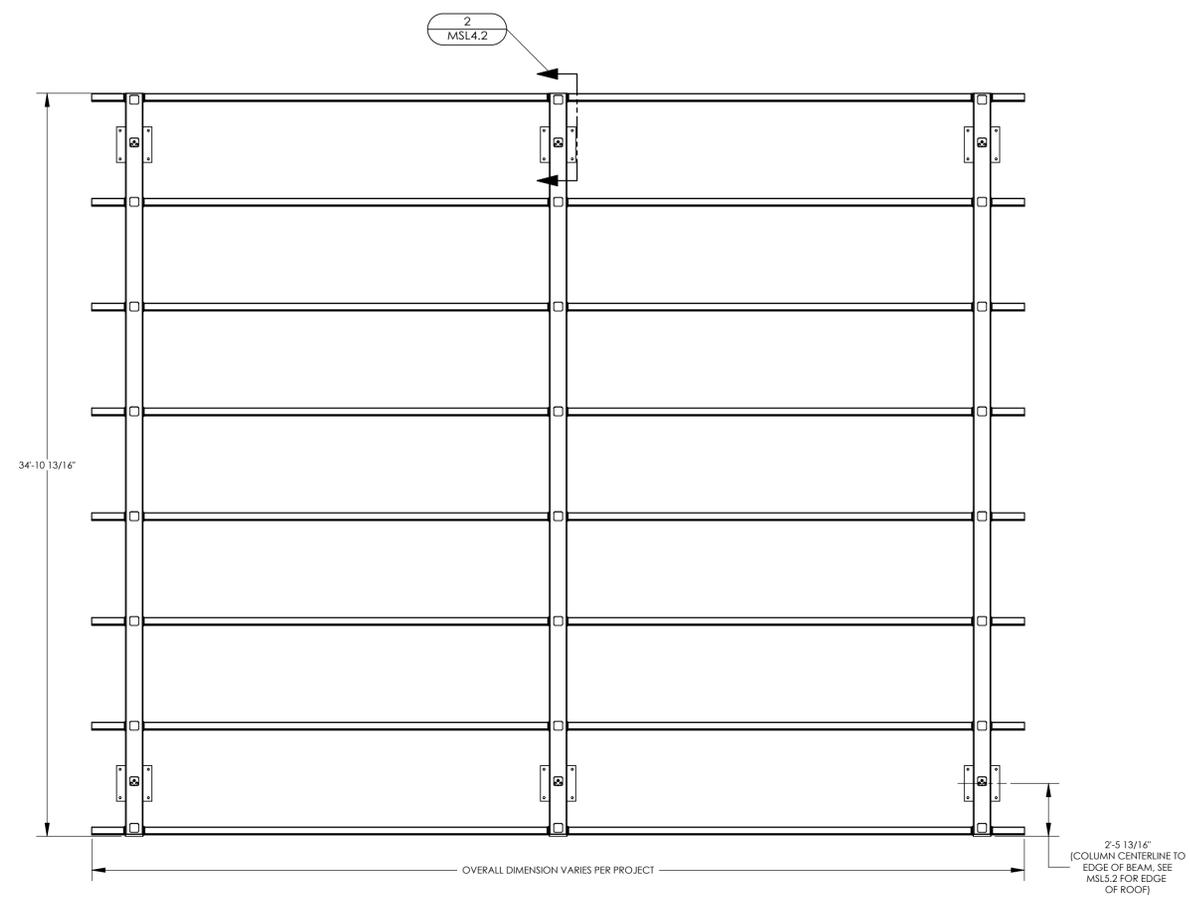
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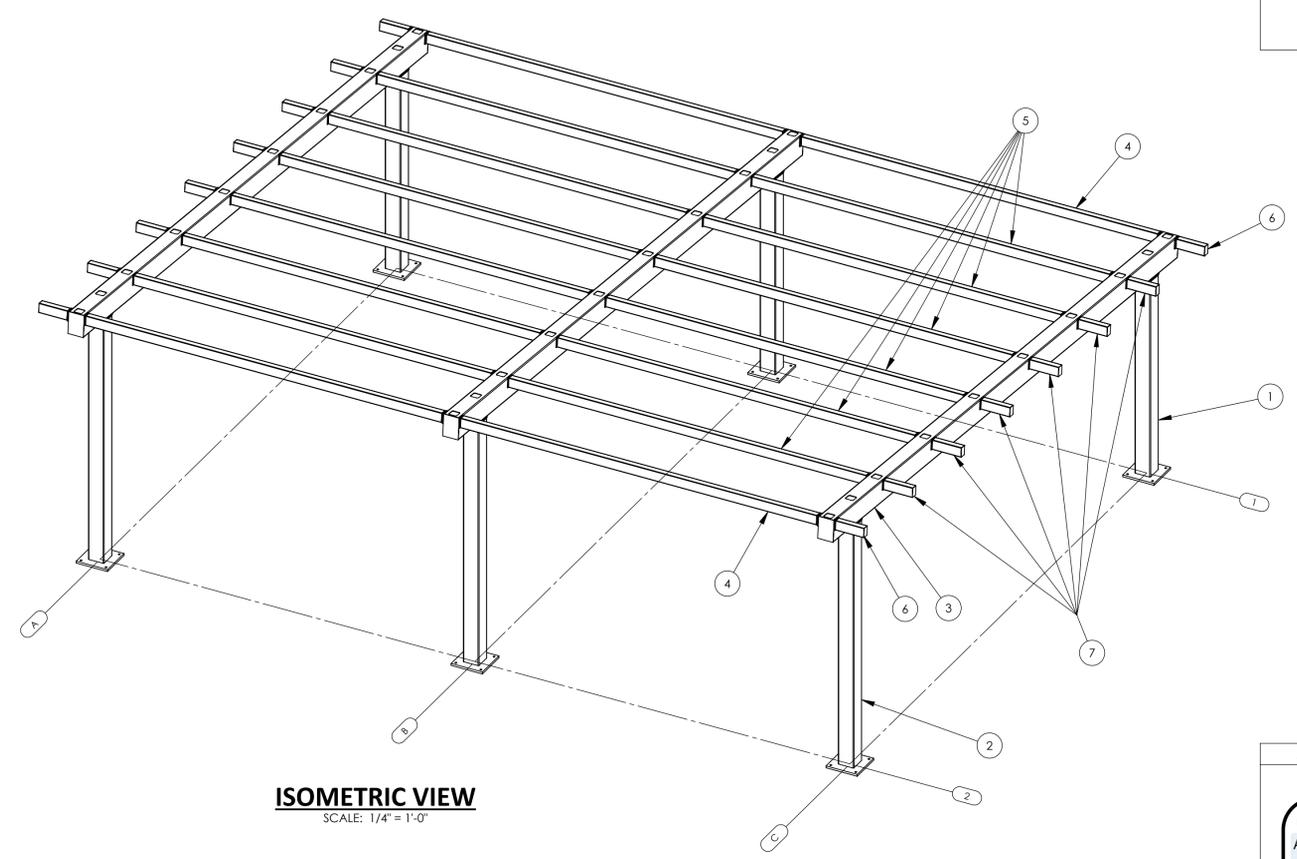
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FRAMING PLAN
MONOSLOPE - MSL 3.5

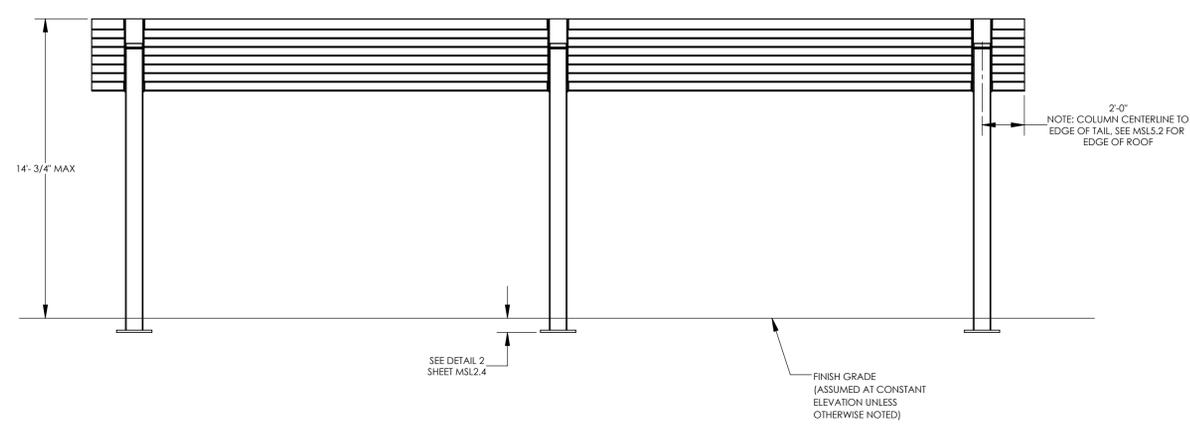
MSL3.2



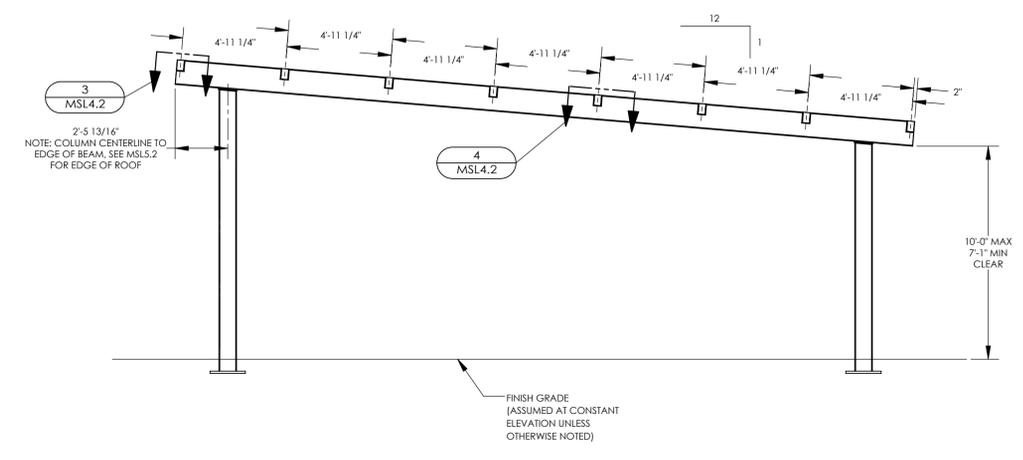
PLAN VIEW
SCALE: 1/4" = 1'-0"



ISOMETRIC VIEW
SCALE: 1/4" = 1'-0"



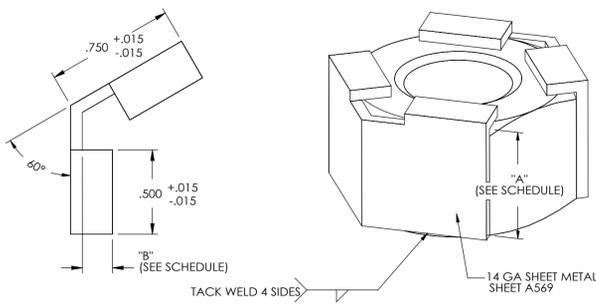
FRONT ELEVATION
SCALE: 1/4" = 1'-0"



SIDE ELEVATION
SCALE: 1/4" = 1'-0"

ITEM	QTY.	PART NO.	DESCRIPTION	MATERIAL	WEIGHT
7	12	-	PURLIN TAIL ASM	HSS6X4X1/8	15.69
6	4	-	EBEAM TAIL ASM	HSS6X4X1/8	16.16
5	12	-	PURLIN ASM	HSS6X4X1/8	159.23
4	4	-	EBEAM ASM	HSS6X4X1/8	160.18
3	3	-	GABLE BEAM_1 ASM	HSS14X10X1/4	1427.07
2	3	-	COL_1 ASM	HSS10X10X1/4	576.19
1	3	-	COL_1 ASM	HSS10X10X1/4	495.13

ALL HIDDEN NUTS AND BOLTS (INSTALLED IN SHOP DURING FABRICATION) ARE SECURED WITH THIS NUT AND BOLT RESTAINING SYSTEM.

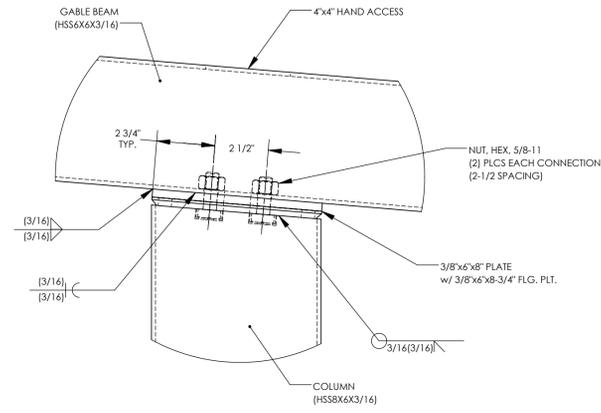


DIMENSION SCHEDULE

FASTENER	DIM A	DIM B
5/8" NUT	.631 +.000 -.015	.180 +.015 -.015
5/8" BOLT	.403 +.000 -.015	.250 +.015 -.015
3/4" NUT	.758 +.000 -.015	.180 +.015 -.015
3/4" BOLT	.483 +.000 -.015	.375 +.015 -.015
1" NUT	1.012 +.000 -.015	.180 +.015 -.015
1" BOLT	.643 +.000 -.015	.375 +.015 -.015

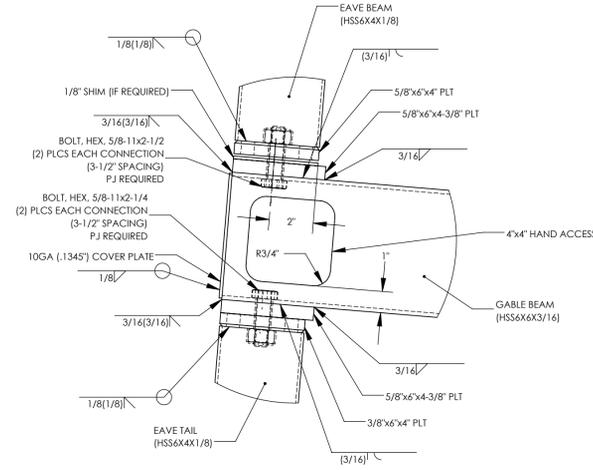
NUT & BOLT RESTRAINING SYSTEM

1



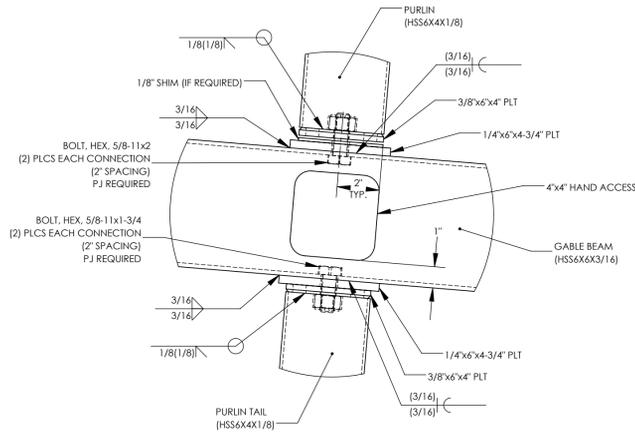
GABLE BEAM CONNECTION @ COLUMN

2



EAVE BEAM AND EAVE TAIL CONNECTIONS

3



PURLIN AND PURLIN TAIL CONNECTIONS

4

STATE APPROVALS-SITE

4099 PLAZA GOLD DRAGON CIRCLE
SUITE B
CAMERON PARK, CA 95002
ESD 677.3515



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STATE APPROVALS-PC

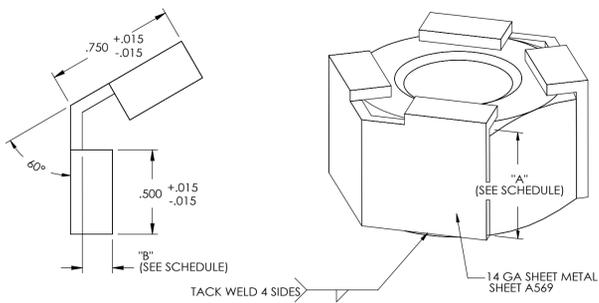
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PRE-CHECK (PC)
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FRAME
CONNECTION
DETAILS
MONOSLOPE - MSL 15

MSL4.0

ALL HIDDEN NUTS AND BOLTS (INSTALLED IN SHOP DURING FABRICATION) ARE SECURED WITH THIS NUT AND BOLT RESTAINING SYSTEM.

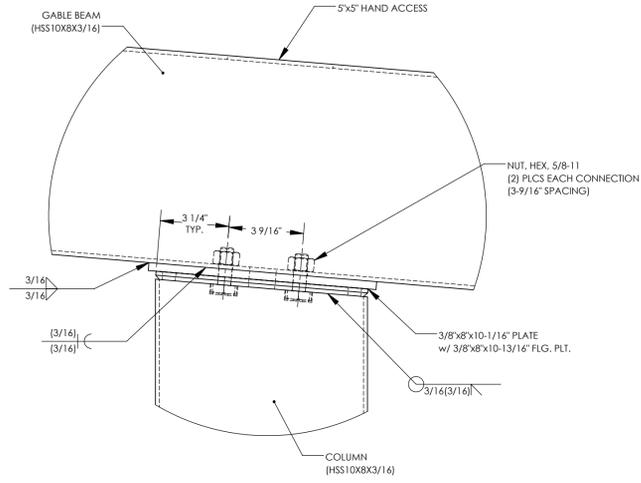


DIMENSION SCHEDULE

FASTENER	DIM A	DIM B
5/8" NUT	.631 +.000 -.015	.180 +.015 -.015
5/8" BOLT	.403 +.000 -.015	.250 +.015 -.015
3/4" NUT	.758 +.000 -.015	.180 +.015 -.015
3/4" BOLT	.483 +.000 -.015	.375 +.015 -.015
1" NUT	1.012 +.000 -.015	.180 +.015 -.015
1" BOLT	.643 +.000 -.015	.375 +.015 -.015

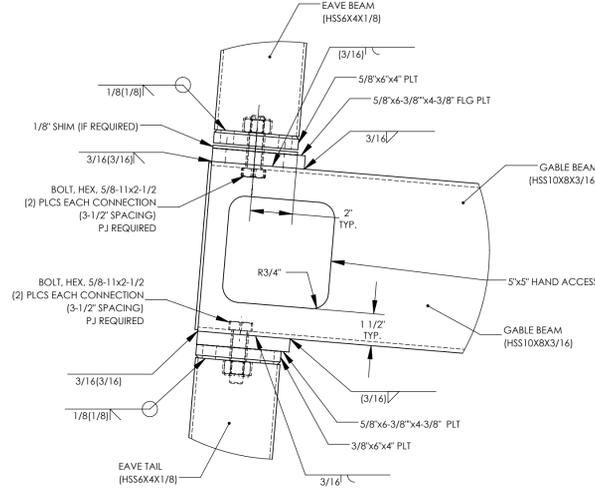
NUT & BOLT RESTRAINING SYSTEM

1



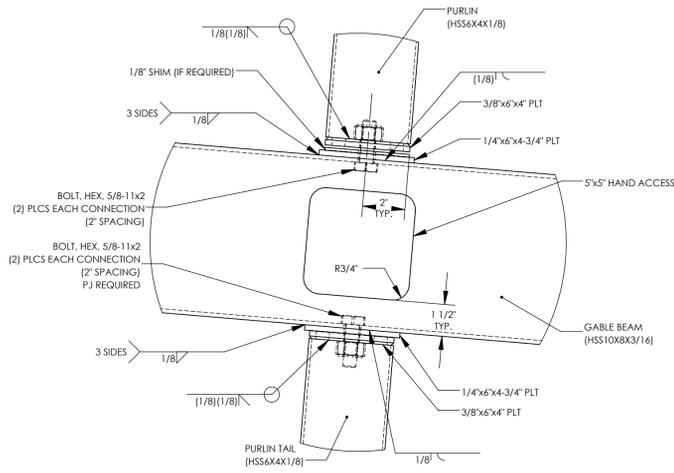
GABLE BEAM CONNECTION @ COLUMN

2



EAVE BEAM AND EAVE TAIL CONNECTIONS

3



PURLIN AND PURLIN TAIL CONNECTIONS

4

STATE APPROVALS-SITE

4099 PLAZA GOLDENADO CIRCLE
SUITE B
CAMERON PARK, CA 95002
ESD 677.3515



poligon
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STATE APPROVALS-PC

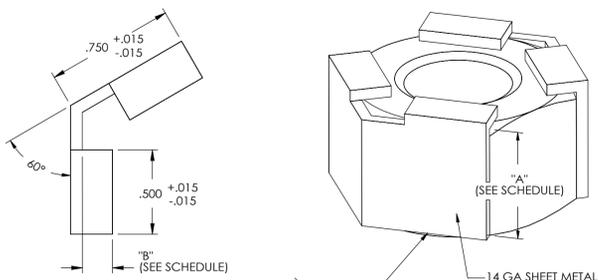
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**PRE-CHECK (PC)
DOCUMENT**
CODE: 2022 CBC
A SEPARATE PROJECT
APPLICATION FOR
CONSTRUCTION IS REQUIRED.

**FRAME
CONNECTION
DETAILS**
MONOSLOPE - MSL 25

MSL4.1

ALL HIDDEN NUTS AND BOLTS (INSTALLED IN SHOP DURING FABRICATION) ARE SECURED WITH THIS NUT AND BOLT RESTAINING SYSTEM.

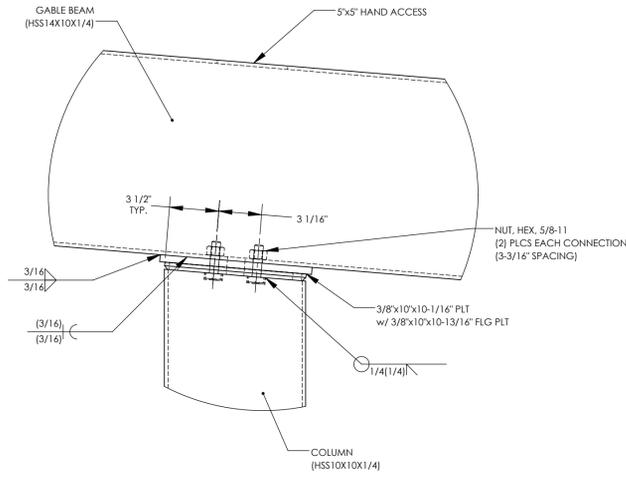


DIMENSION SCHEDULE

FASTENER	DIM A	DIM B
5/8" NUT	.631 +.000 -.015	.180 +.015 -.015
5/8" BOLT	.403 +.000 -.015	.250 +.015 -.015
3/4" NUT	.758 +.000 -.015	.180 +.015 -.015
3/4" BOLT	.483 +.000 -.015	.375 +.015 -.015
1" NUT	1.012 +.000 -.015	.180 +.015 -.015
1" BOLT	.643 +.000 -.015	.375 +.015 -.015

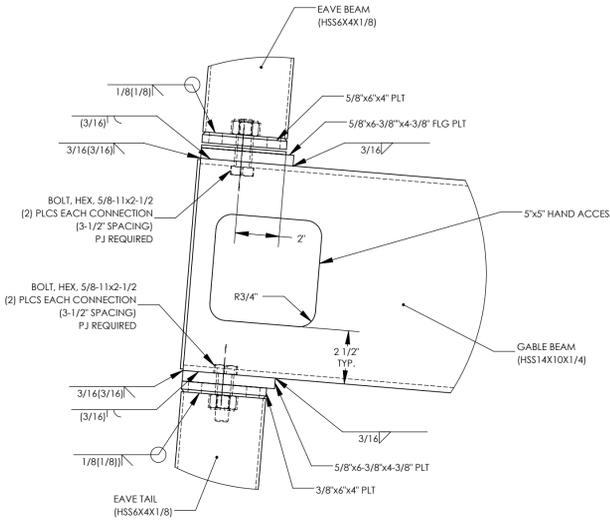
NUT & BOLT RESTRAINING SYSTEM

1



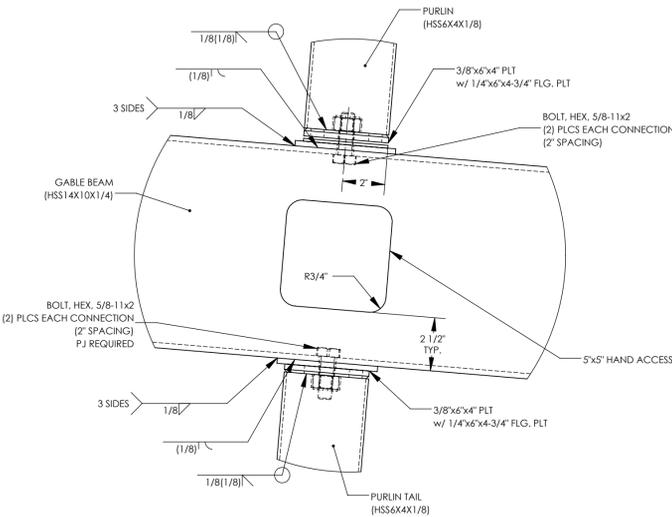
GABLE BEAM CONNECTION @ COLUMN

2



EAVE BEAM AND EAVE TAIL CONNECTIONS

3



PURLIN AND PURLIN TAIL CONNECTIONS

4

STATE APPROVALS-SITE

4099 PLAZA GOLDENADO CIRCLE
SUITE B
CAMERON PARK, CA 95902
ESD 677.3515



poligon
PORTER
A POLYGON COMPANY



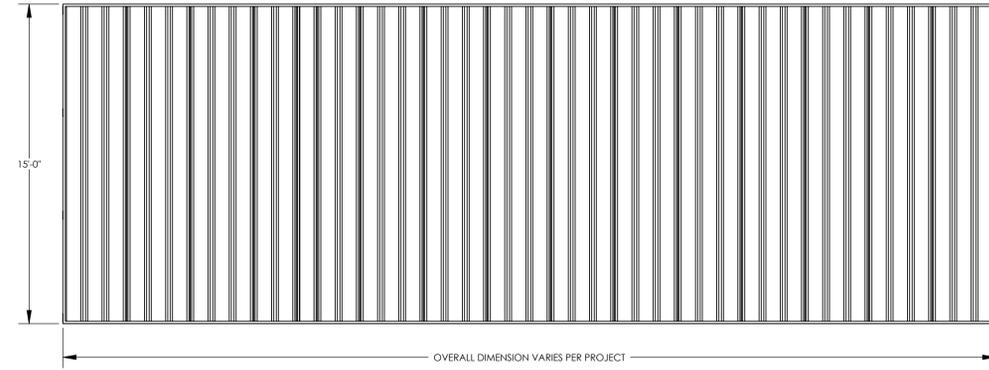
STATE APPROVALS-PC



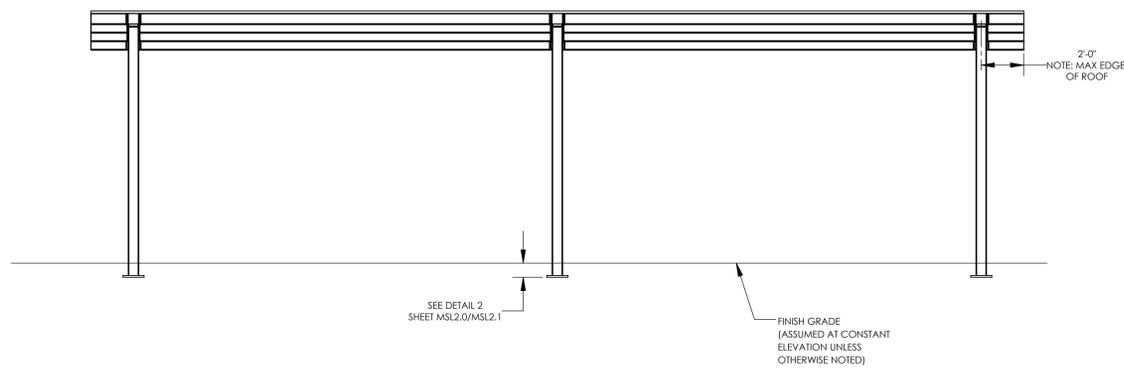
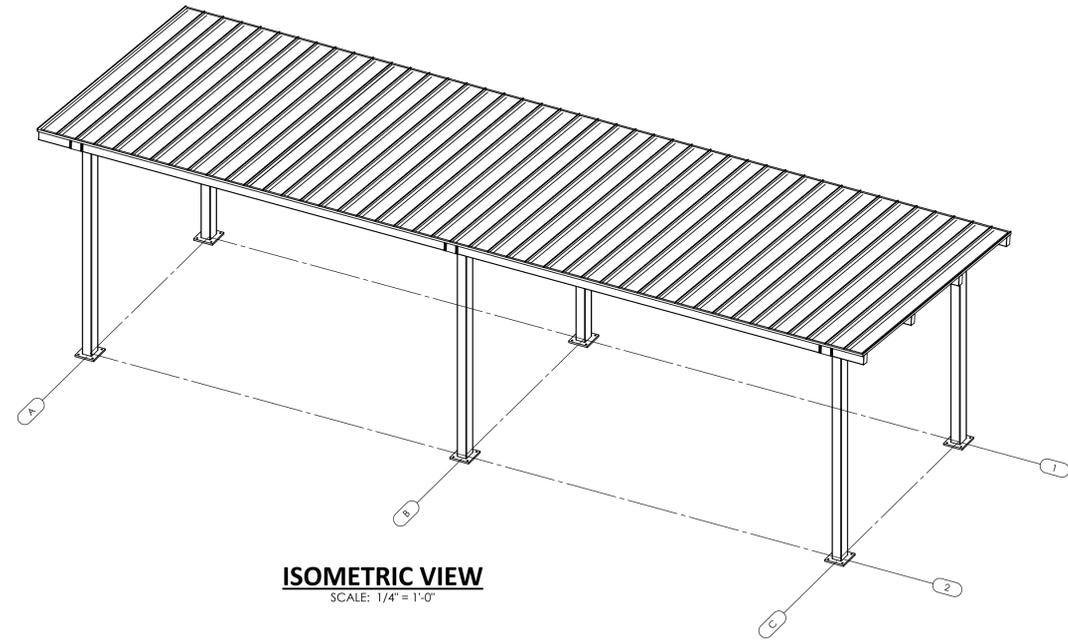
PRE-CHECK (PC) DOCUMENT
CODE: 2022 CBC
A SEPARATE PROJECT APPLICATION FOR CONSTRUCTION IS REQUIRED.

FRAME CONNECTION DETAILS
MONOSLOPE - MSL 35

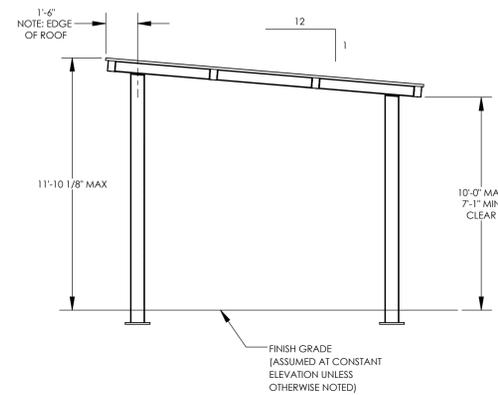
MSL4.2



PLAN VIEW
SCALE: 1/4" = 1'-0"



FRONT ELEVATION
SCALE: 1/4" = 1'-0"



SIDE ELEVATION
SCALE: 1/4" = 1'-0"

STATE APPROVALS-SITE

4699 PLAZA GOLDEN GATE CIRCLE
SUITE B
CAMERON PARK, CA 95002
530.677.3515



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STATE APPROVALS-PC

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-121212 PC
REVIEWED FOR
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DATE: 7/18/2023

**PRE-CHECK (PC)
DOCUMENT**
CODE: 2022 CBC
A SEPARATE PROJECT
APPLICATION FOR
CONSTRUCTION IS REQUIRED.

ARCHITECTURAL
VIEWS

MONOSLOPE - MSL 1.5

MSL5.0

STATE APPROVALS-SITE

4690 PLAZA GOLDEN GATE CIRCLE
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CAMERON PARK, CA 95002
530.677.3515



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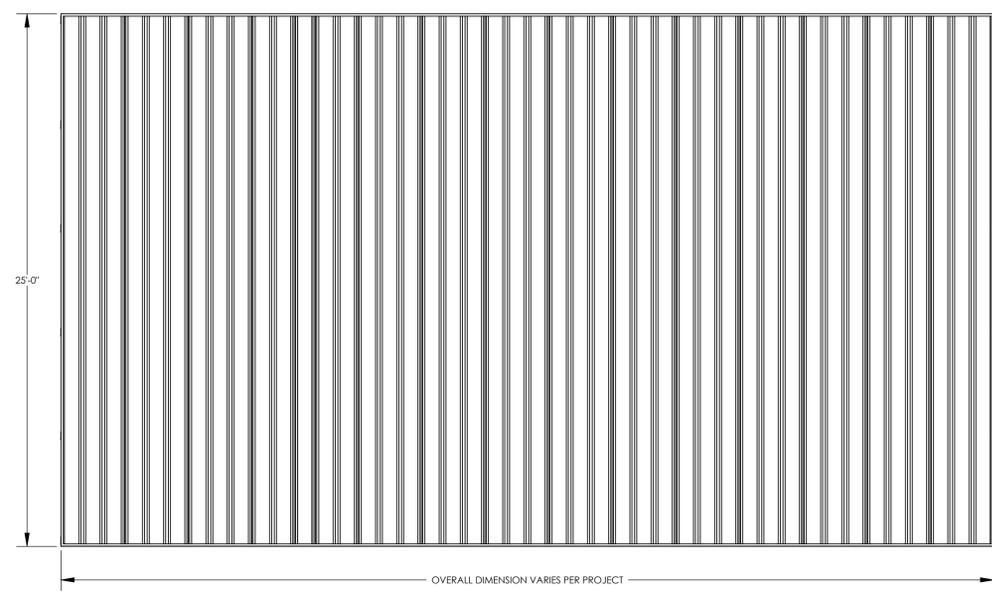


STATE APPROVALS-PC
IDENTIFICATION STAMP
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APP: 02-121212 PC
REVIEWED FOR
SS FLS ACS CG
DATE: 7/18/2023

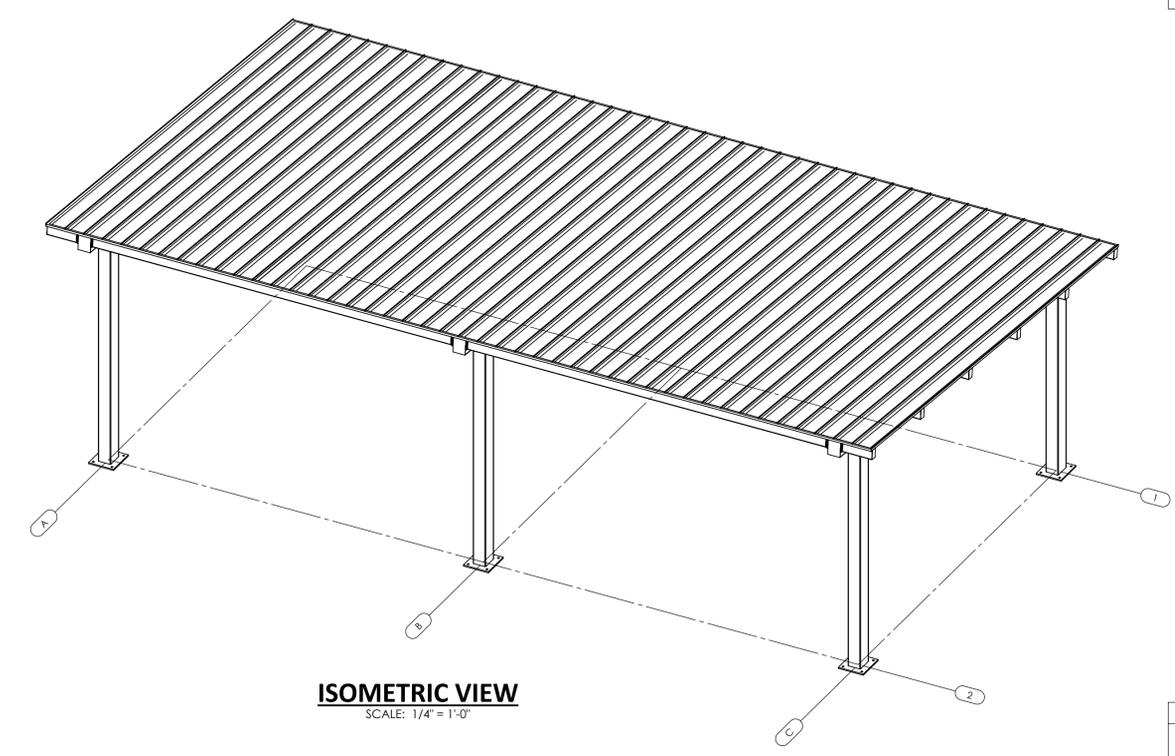
**PRE-CHECK (PC)
DOCUMENT**
CODE: 2022 CBC
A SEPARATE PROJECT
APPLICATION FOR
CONSTRUCTION IS REQUIRED.

ARCHITECTURAL
VIEWS
MONOSLOPE - MSL 25

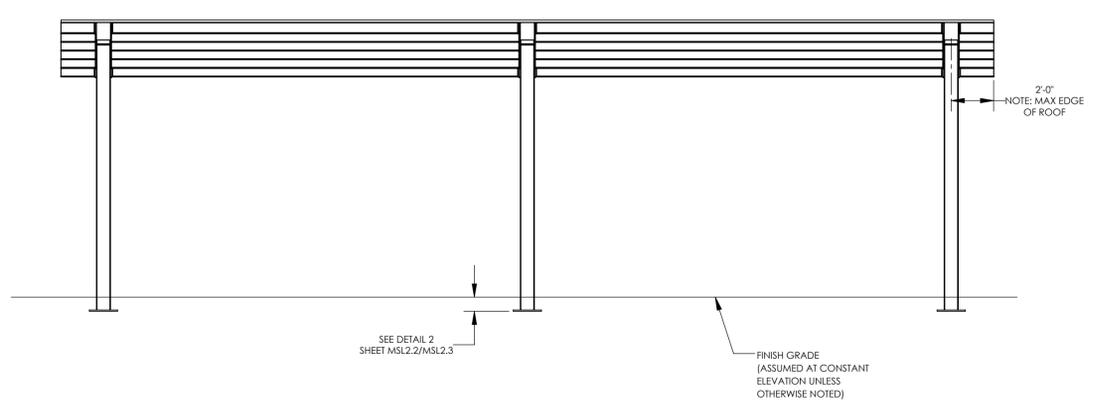
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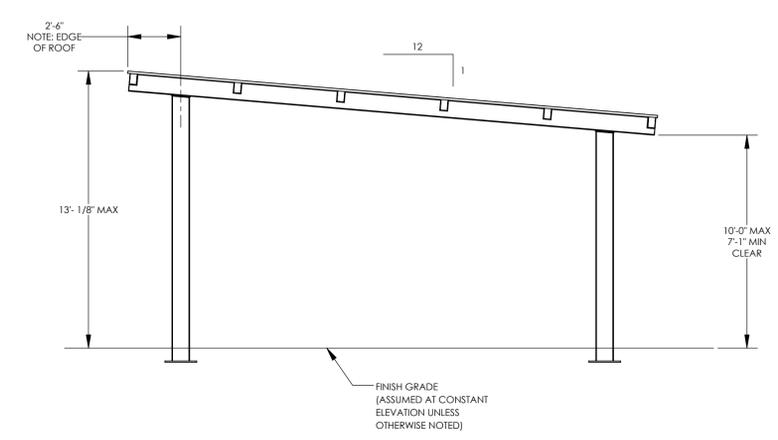
PLAN VIEW
SCALE: 1/4" = 1'-0"



ISOMETRIC VIEW
SCALE: 1/4" = 1'-0"



FRONT ELEVATION
SCALE: 1/4" = 1'-0"



SIDE ELEVATION
SCALE: 1/4" = 1'-0"

STATE APPROVALS-SITE

4090 PLAZA GOLDEN GATE CIRCLE
SUITE B
CAMERON PARK, CA 95002
530.677.3515



poligon PORTER
A POLYCOM SYSTEM

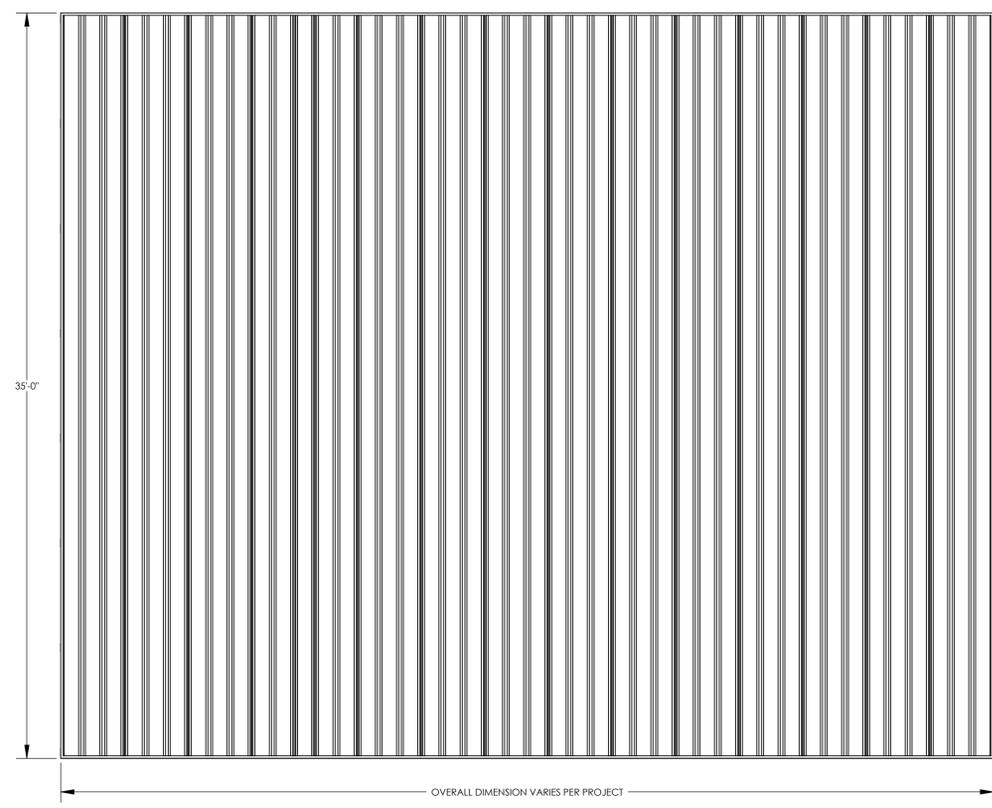


STATE APPROVALS-PC
IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-121212 PC
REVIEWED FOR
SS FLS ACS CG
DATE: 7/18/2023

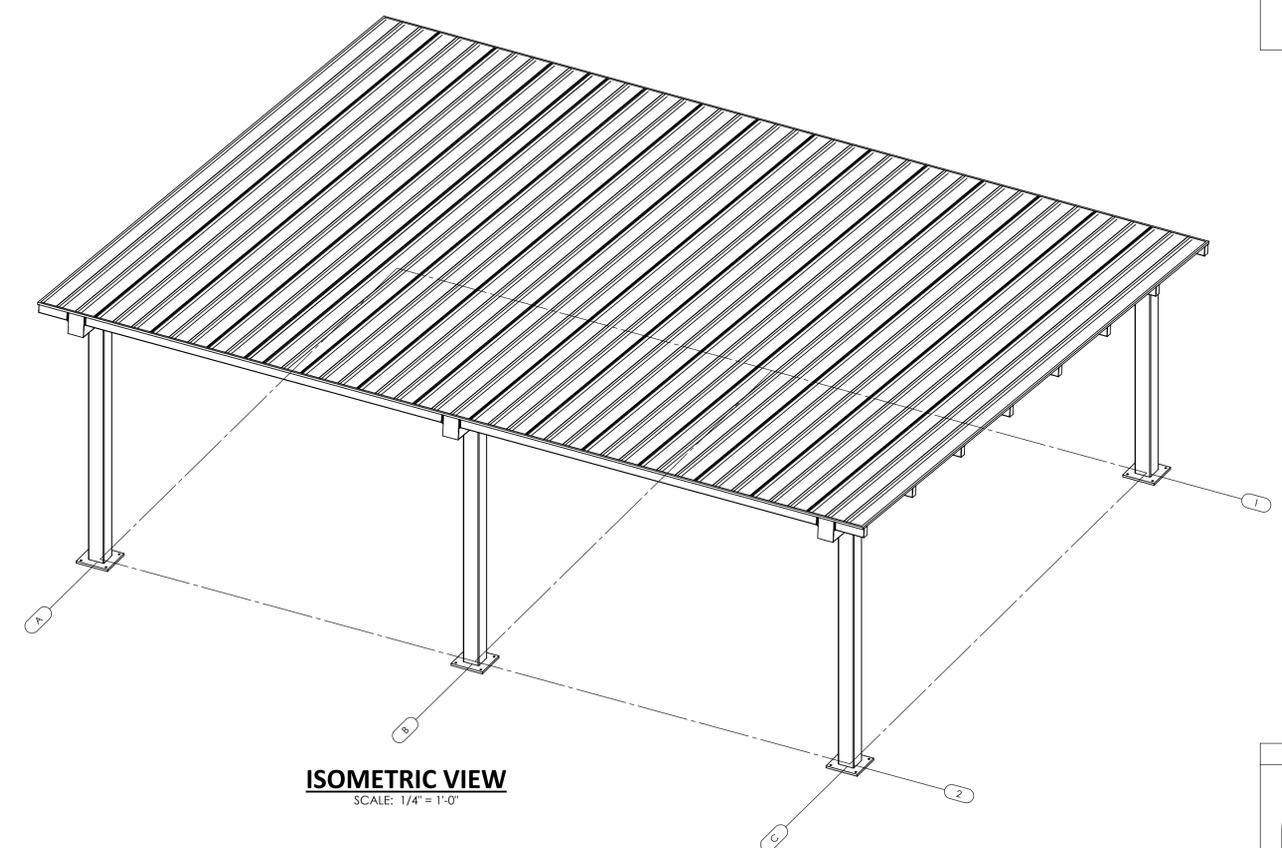
PRE-CHECK (PC)
DOCUMENT
CODE: 2022 CBC
A SEPARATE PROJECT
APPLICATION FOR
CONSTRUCTION IS REQUIRED.

ARCHITECTURAL
VIEWS
MONOSLOPE - MSL 35

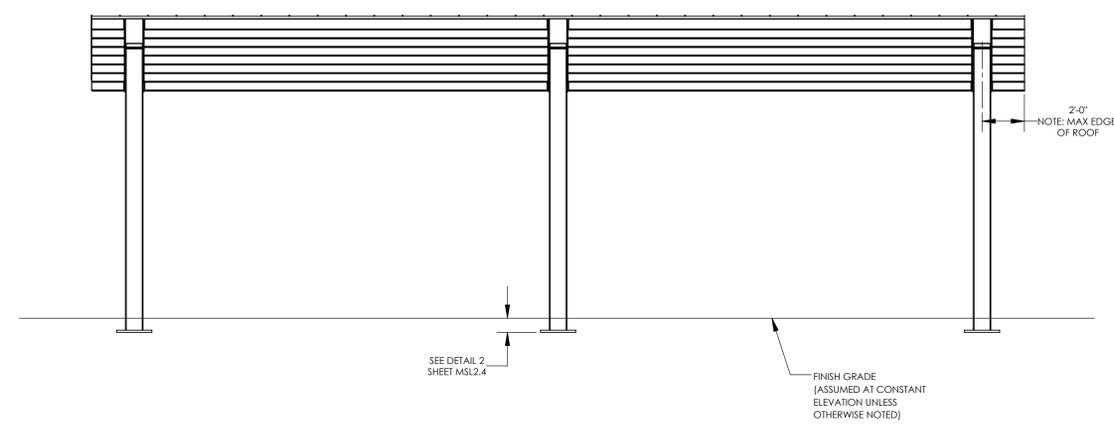
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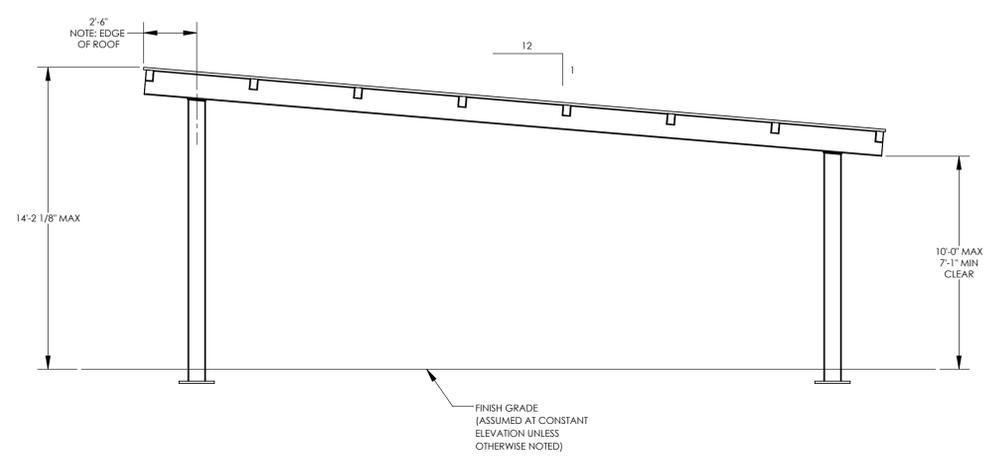
PLAN VIEW
SCALE: 1/4" = 1'-0"



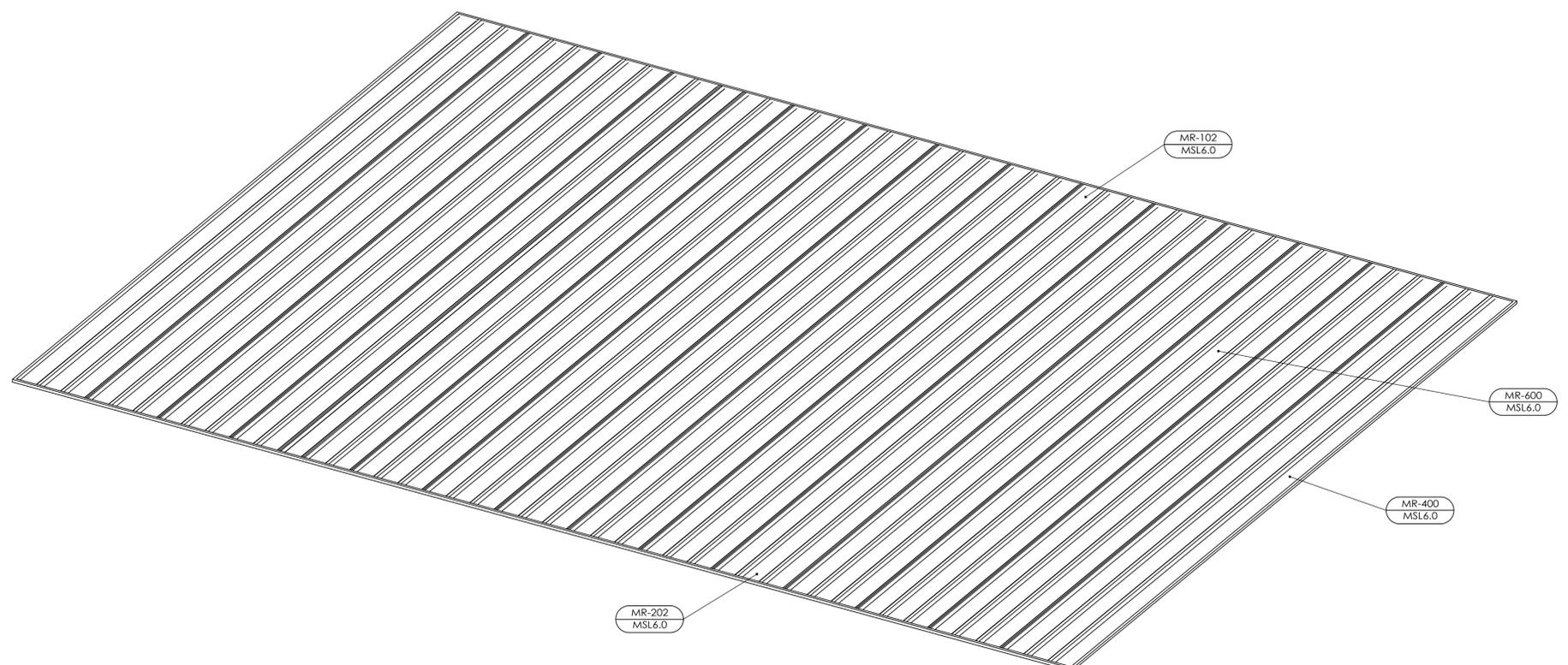
ISOMETRIC VIEW
SCALE: 1/4" = 1'-0"



FRONT ELEVATION
SCALE: 1/4" = 1'-0"



SIDE ELEVATION
SCALE: 1/4" = 1'-0"



MULTI-RIB NOTES:

THE DETAILS SHOWN ARE SUGGESTIONS OR GUIDELINES ON HOW TO ERECT THE SYSTEMS. THE INFORMATION SHOWN IS ACCURATE, BUT IT IS NOT INTENDED TO COVER ALL INSTANCES, BUILDING REQUIREMENTS, DESIGNS OR CODES. THE DETAILS MAY REQUIRE CHANGES OR REVISIONS DUE TO FIELD CONDITIONS.

IT SHALL BE THE RESPONSIBILITY OF THE ERECTOR TO ENSURE THAT THE DETAILS MEET PARTICULAR BUILDING REQUIREMENTS AND TO ASSURE ADEQUATE WATER TIGHTNESS.

THE ERECTOR SHOULD THOROUGHLY FAMILIARIZE HIMSELF/HERSELF WITH ALL ERECTION INSTRUCTIONS BEFORE STARTING WORK.

THE PANELS SHOULD BE INSTALLED PLUMB, STRAIGHT, AND ACCURATELY TO THE ADJACENT WORK.

FLASHING AND TRIM SHALL BE INSTALLED TRUE, AND IN PROPER ALIGNMENT, WITH ANY EXPOSED FASTENERS EQUALLY SPACED FOR THE BEST APPEARANCE.

SEALANT SHALL BE FIELD APPLIED ON DRY, CLEAN SURFACES. SOME FIELD CUTTING AND FITTING OF PANELS AND FLASHING IS TO BE EXPECTED BY THE ERECTOR AND MINOR FIELD CORRECTIONS ARE A PART OF NORMAL ERECTION WORK.

WORKMANSHIP SHALL BE OF THE BEST INDUSTRY STANDARDS AND INSTALLATION SHALL BE PERFORMED BY EXPERIENCED METAL CRAFTSMEN.

METAL SHAVINGS FROM DRILLING OR INSTALLATION OF ROOF FASTENERS MUST BE CAREFULLY REMOVED FROM THE ROOF BY BRUSHING OR SWEEPING AT THE END OF EACH DAY DURING INSTALLATION. SHAVINGS LEFT ON THE ROOF WILL QUICKLY RUST AND STAIN THE ROOF FINISH.

COVER ACCESS HOLES WITH GRACE ICE AND WATER SHIELD BEFORE ATTACHING ROOF DECK.

METAL ROOFING PRODUCT AND INSTALLATION SHALL MEET ALL REQUIREMENTS OF UL 580.

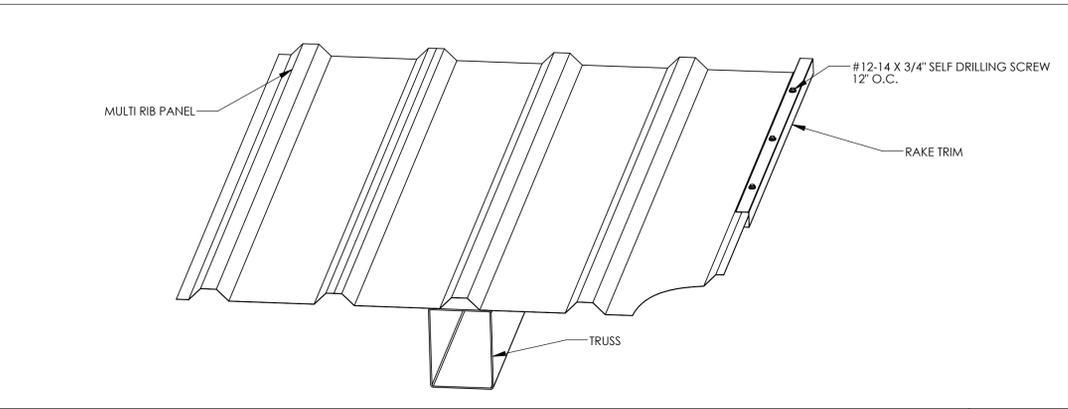
STATE APPROVALS-SITE

4699 PLAZA GOLDEN GATE CIRCLE
SUITE B
CAMERON PARK, CA 95002
530.677.3515

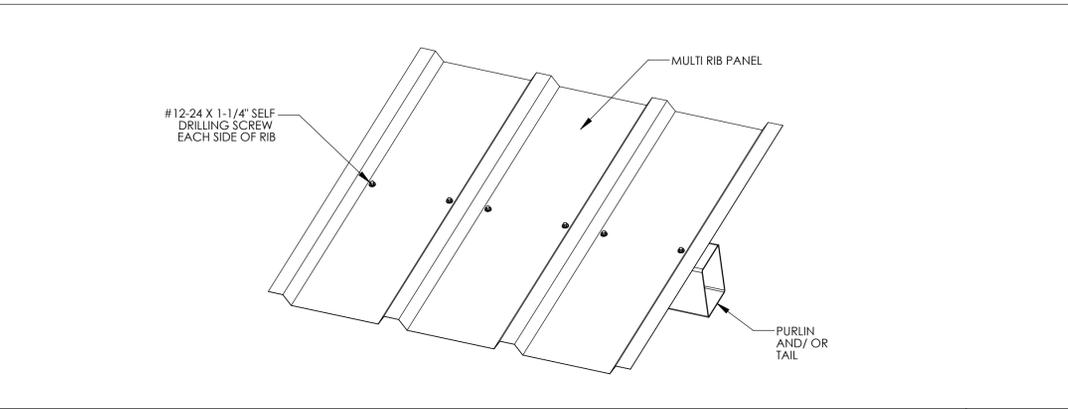
poligon PORTER
A HFC COMPANY

STATE APPROVALS-PC

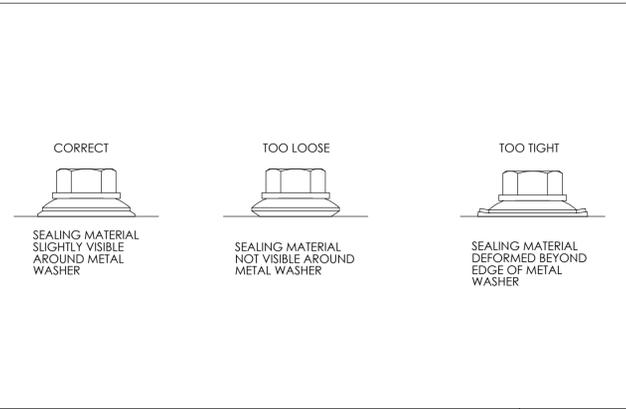
IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-121212 PC
REVIEWED FOR
SS FLS ACS CG
DATE: 7/18/2023



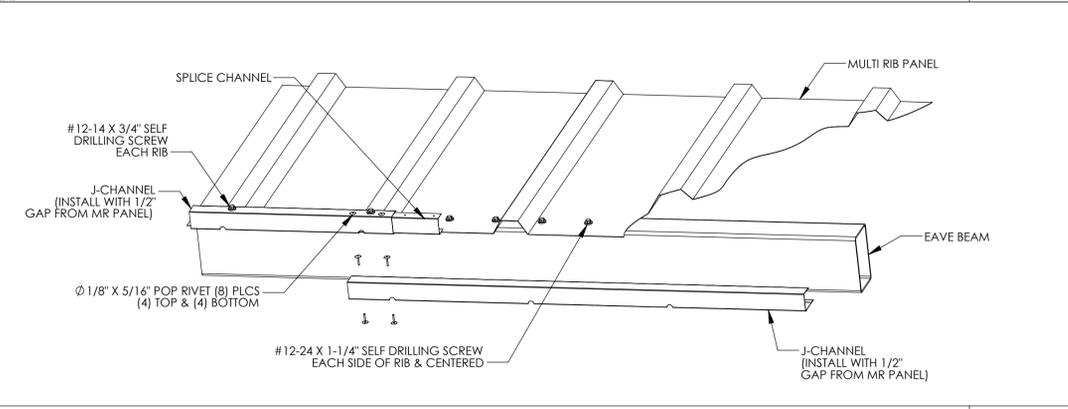
RAKE DETAIL MR-400



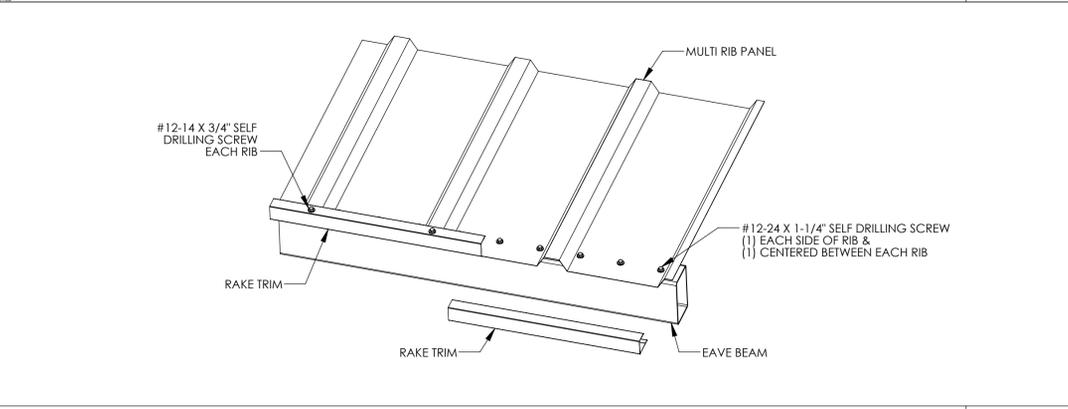
PURLIN DETAIL MR-600



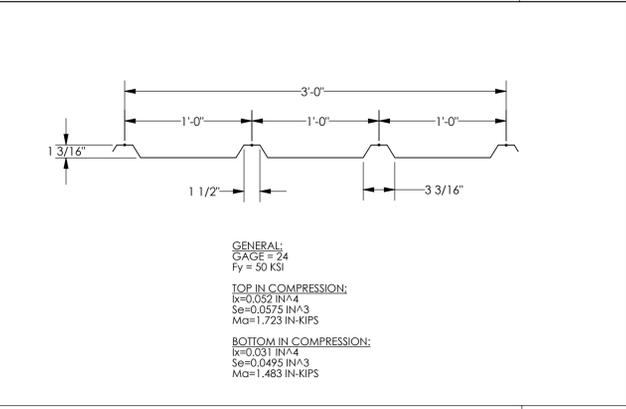
ROOF FASTENER TIGHTENING MR-950



EAVE DETAIL MR-102



HIGHSIDE DETAIL (VIEWING FROM ABOVE) MR-202

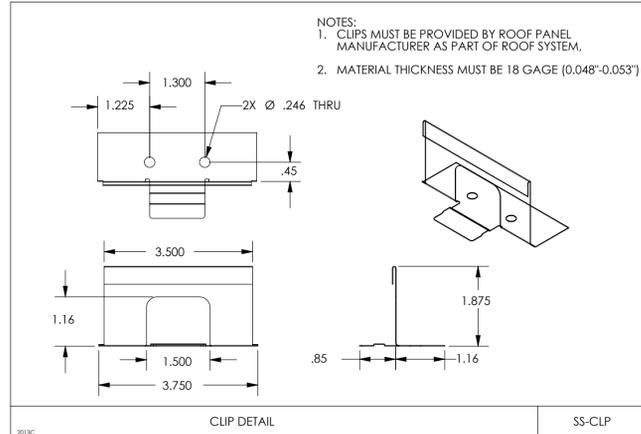
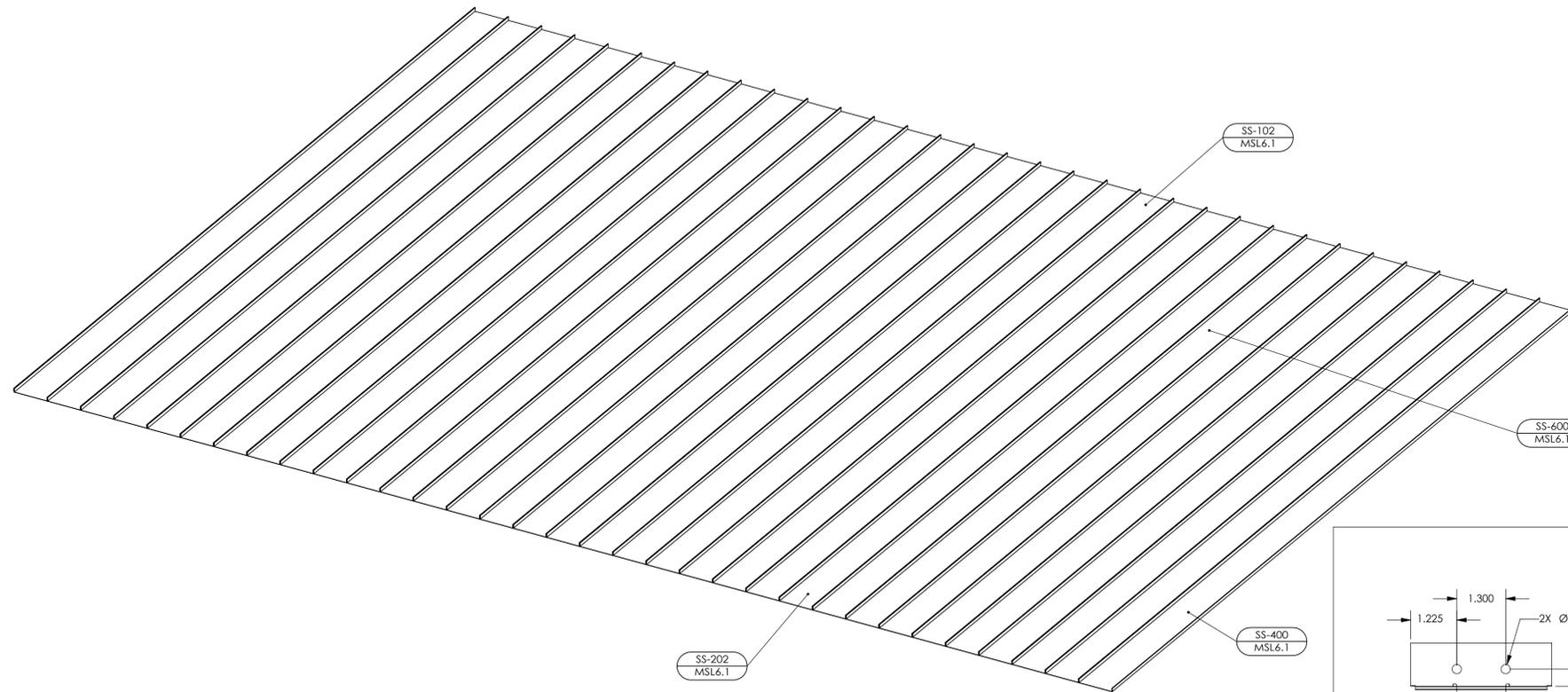


MR ROOF DECK SECTION PROPERTIES MR-951

PRE-CHECK (PC)
DOCUMENT
CODE: 2022 CBC
A SEPARATE PROJECT
APPLICATION FOR
CONSTRUCTION IS REQUIRED.

ROOF CONNECTION
DETAILS
MONOSLOPE - MSL

MSL6.0



STANDING SEAM INSTALLATION NOTES:

THE DETAILS SHOWN ARE SUGGESTIONS OR GUIDELINES ON HOW TO ERECT THE SYSTEMS. THE INFORMATION SHOWN IS ACCURATE, BUT IT IS NOT INTENDED TO COVER ALL INSTANCES, BUILDING REQUIREMENTS, DESIGNS OR CODES. THE DETAILS MAY REQUIRE CHANGES OR REVISIONS DUE TO FIELD CONDITIONS.

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THE ERECTOR SHOULD THOROUGHLY FAMILIARIZE HIMSELF/HERSELF WITH ALL ERECTION INSTRUCTIONS BEFORE STARTING WORK.

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WORKMANSHIP SHALL BE OF THE BEST INDUSTRY STANDARDS AND INSTALLATION SHALL BE PERFORMED BY EXPERIENCED METAL CRAFTSMEN.

METAL SHAVINGS FROM DRILLING OR INSTALLATION OF ROOF FASTENERS MUST BE CAREFULLY REMOVED FROM THE ROOF BY BRUSHING OR SWEEPING AT THE END OF EACH DAY DURING INSTALLATION. SHAVINGS LEFT ON THE ROOF WILL QUICKLY RUST AND STAIN THE ROOF FINISH.

COVER ACCESS HOLES WITH GRACE ICE AND WATER SHIELD BEFORE ATTACHING ROOF DECK.

METAL ROOFING PRODUCT AND INSTALLATION SHALL MEET ALL REQUIREMENTS OF ICC-ES REPORT ESL-1082.

STATE APPROVALS-SITE

4690 PLAZA GOLDEN GATE CIRCLE
 SUITE B
 CAMERON PARK, CA 95002
 530.677.3515

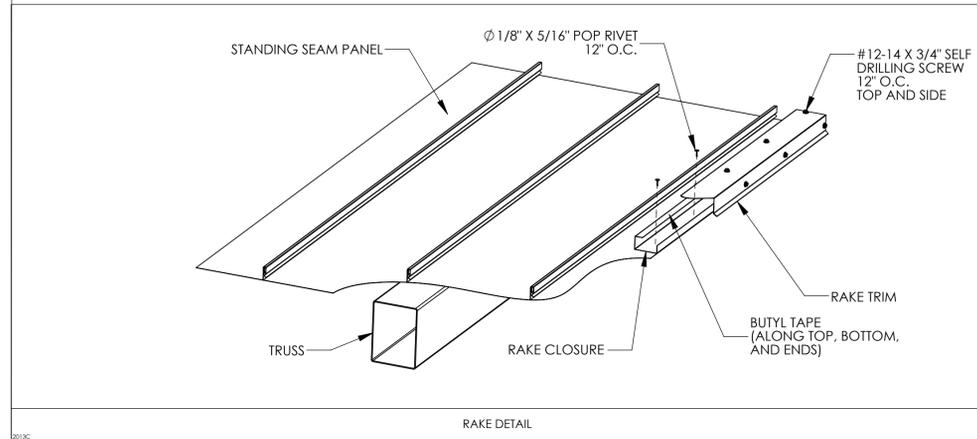
GHD

poligon PORTER
 A HFC COMPANY

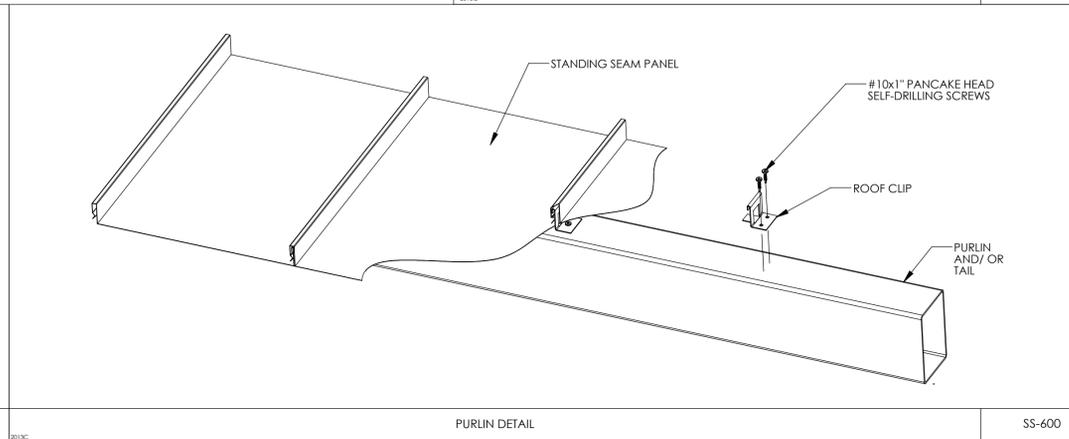
REGISTERED PROFESSIONAL ENGINEER
 JESSICA E. MAPPER
 No. 55476
 STRUCTURAL
 STATE OF CALIFORNIA

STATE APPROVALS-PC

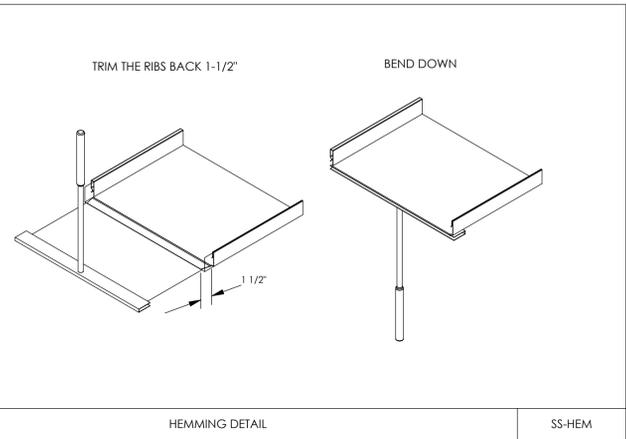
IDENTIFICATION STAMP
 DIV. OF THE STATE ARCHITECT
 APP: 02-121212 PC
 REVIEWED FOR
 SS FLS ACS CG
 DATE: 7/18/2023



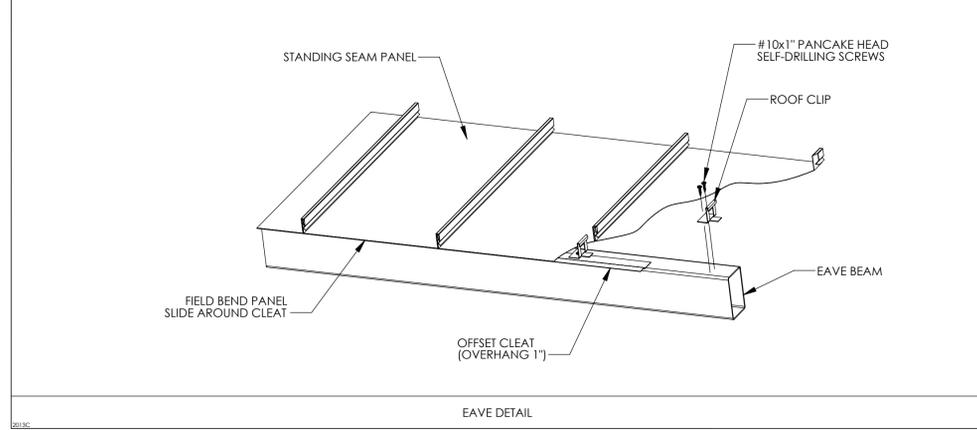
RAKE DETAIL SS-400



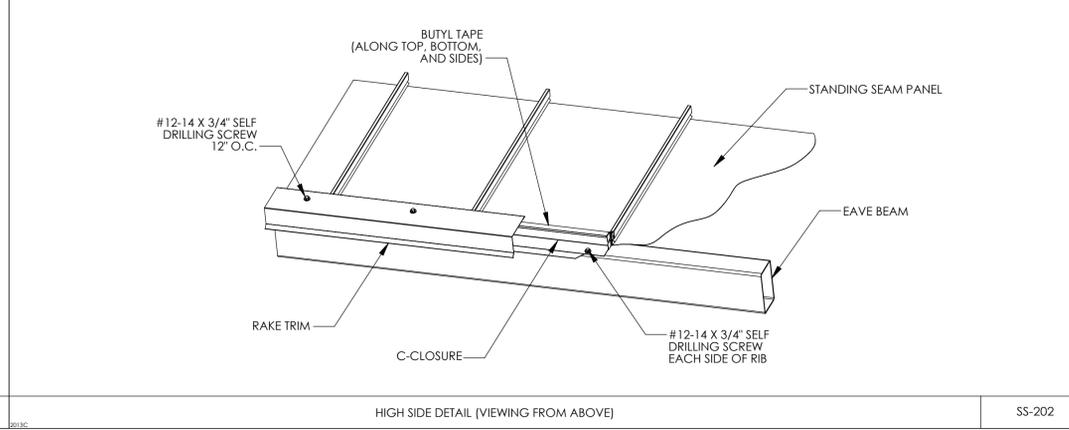
PURLIN DETAIL SS-600



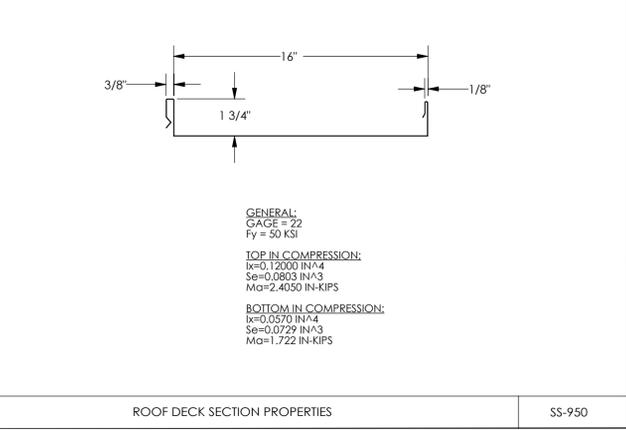
HEMMING DETAIL SS-HEM



EAVE DETAIL SS-102



HIGH SIDE DETAIL (VIEWING FROM ABOVE) SS-202



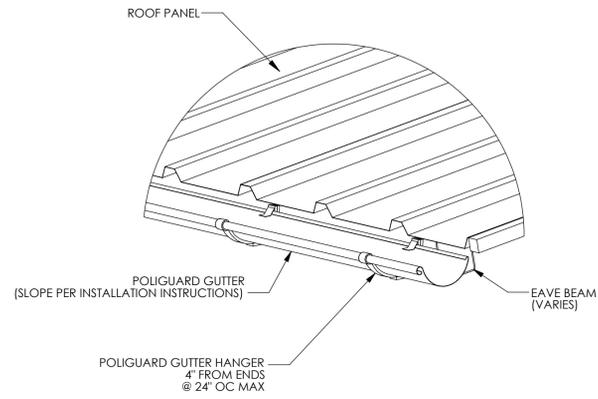
ROOF DECK SECTION PROPERTIES SS-950

PRE-CHECK (PC) DOCUMENT
 CODE: 2022 CBC
 A SEPARATE PROJECT APPLICATION FOR CONSTRUCTION IS REQUIRED.

ROOF CONNECTION DETAILS

MONOSLOPE - MSL

MSL6.1

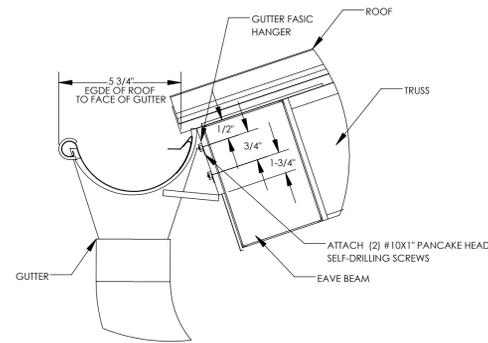


GUTTER DETAIL

GS-100

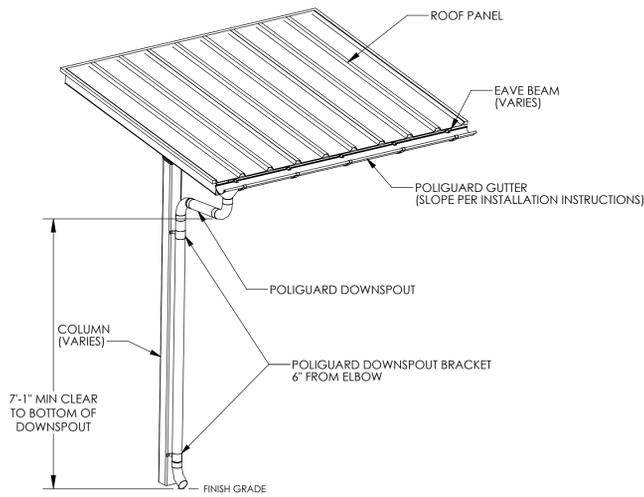
POLIGUARD GUTTER SYSTEM NOTES:

1. PREFABRICATED GUTTER SYSTEM IS ATTACHED TO THE STRUCTURE AFTER ROOF IS INSTALLED.
2. DETAILED INSTALLATION INSTRUCTIONS ARE SHIPPED WITH THE STRUCTURE.
3. DOWNSPOUTS REQUIRED AT EACH COLUMN.



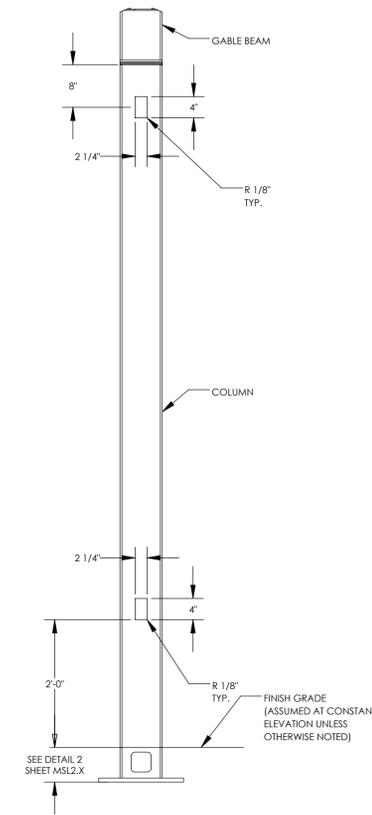
GUTTER DETAIL

GS-200



DOWNSPOUT DETAIL

GS-300



ELECTRICAL CUTOUT IN COLUMNS

EC-100

ELECTRICAL CUTOOUT NOTES:

1. MAXIMUM ONE CUTOOUT PERMITTED IN EACH MEMBER.
2. CUTOOUTS CAN BE PLACED ON ANY SIDE OF A MEMBER.
3. CUTOOUTS CAN BE PLACED ALONG MEMBERS AS INDICATED IN THE DETAILS.
4. ARCHITECTS REQUESTING CUTOOUTS MUST MARKUP APPROVED PC DRAWINGS TO LOCATE CUTOOUTS FOR APPROVAL AND FABRICATION.

STATE APPROVALS-SITE

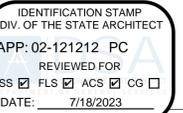
4699 PLAZA GOLDEN GATE CIRCLE
SUITE B
CAMERON PARK, CA 95002
ESD 677.3515



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A POLYCOR INC. COMPANY



STATE APPROVALS-PC



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MISC DESIGN OPTIONS
MONOSLOPE - MSL

MSL7.0

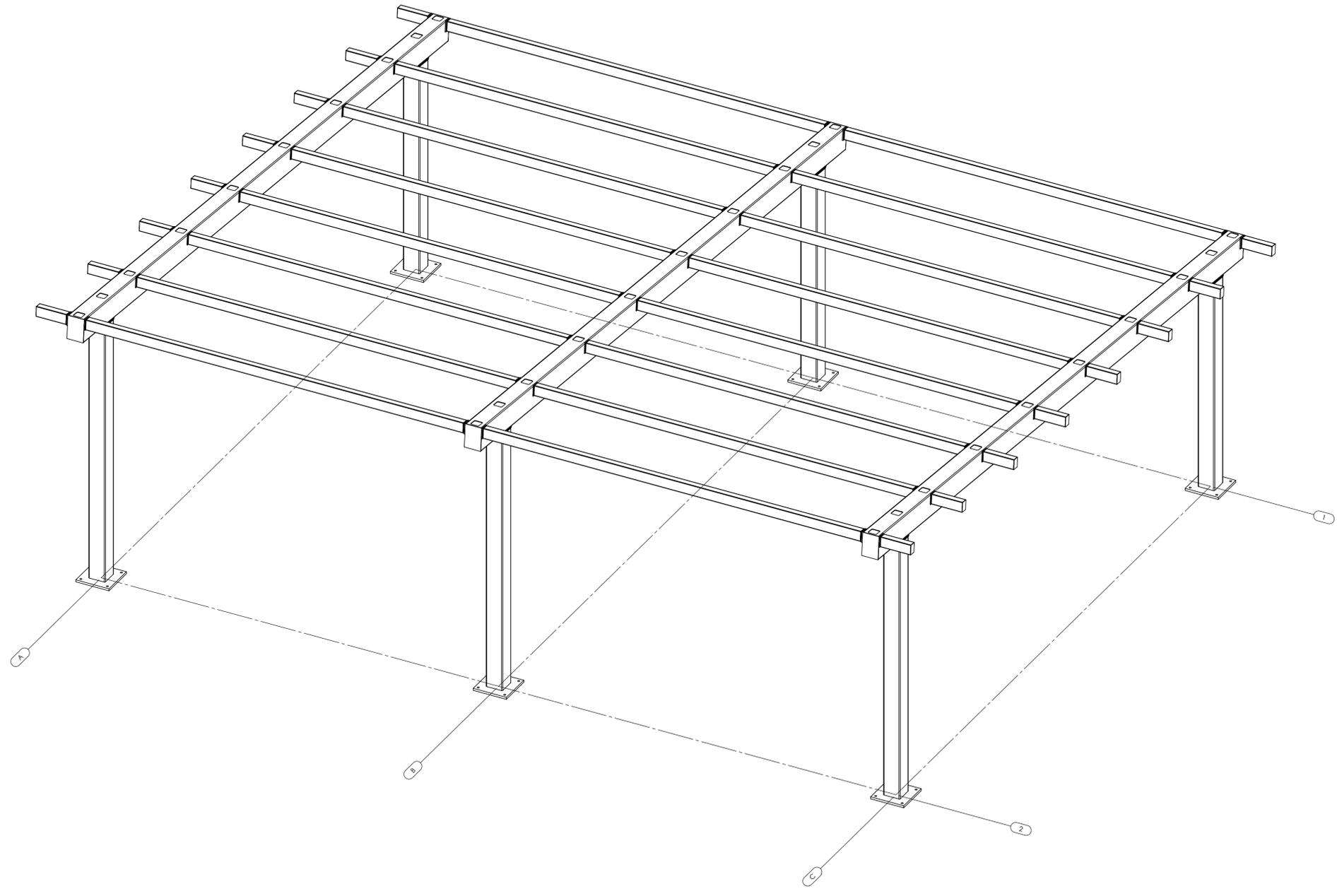
ELECTRICAL CUTOUT AND ACCESS INSTRUCTIONS

- IF 'YES' IS NOT SELECTED IN STEP 2 ON ORDER FORM, THEN THIS SHEET NEED NOT BE INCLUDED IN SITE-SPECIFIC DRAWINGS
- ONLY COLUMNS ARE PERMITTED TO HAVE ELECTRICAL ACCESS
- THE COLUMN CUTOUTS ARE STATIC AND SHOWN IN THE 'MSC DESIGN OPTIONS SHEET'
- IDENTIFY THE COLUMNS WITH ELECTRICAL CUTOUTS BELOW (REFERENCE GRID LINES IN ISOMETRIC FRAME VIEW TO THE RIGHT)
- STRUCTURES MAY BE LONGER OR SHORTER THAN THE ISOMETRIC FRAME VIEW SHOWN
- IF SITE-SPECIFIC STRUCTURE HAS A DIFFERENT NUMBER OF COLUMNS THAN ISOMETRIC SHOWN, REFERENCE COLUMN A1 IN THE ISOMETRIC VIEW AND CONTINUE PATTERN TO FIT SITE-SPECIFIC LAYOUT
- IF NO COLUMNS ARE IDENTIFIED, POLIGON WILL ASSUME CUTOUTS ONLY IN COLUMN A1
- CONTACT POLIGON ENGINEERING FOR SPECIAL PROJECT SPECIFIC REQUIREMENTS

ELECTRICAL CUTOUT IDENTIFICATION IN COLUMNS	
SPECIFIC MEMBERS	_____

EXAMPLE:

ELECTRICAL CUTOUT IDENTIFICATION IN COLUMNS	
SPECIFIC MEMBERS	A1, B1, F1



STATE APPROVALS-SITE

4690 PLAZA GOLDEN GATE CIRCLE
SUITE B
CAMERON PARK, CA 95002
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PORTER CONSTRUCTION



STATE APPROVALS-PC

IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT APP: 02-121212 PC REVIEWED FOR SS <input checked="" type="checkbox"/> FLS <input checked="" type="checkbox"/> ACS <input checked="" type="checkbox"/> CG <input type="checkbox"/> DATE: 7/18/2023

**PRE-CHECK (PC)
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**ELECTRICAL
CUTOUTS**
MONOSLOPE - MSL

MSL7.1